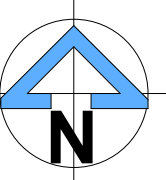


			CLIENT: COOTAMUNDRA GUNDAGAI REGIONAL COUNCIL	ARCHITECT: Noel Thomson Architecture Pty Ltd.	PROJECT NO:	NOEL THOMSON ARCHITECTURE PTY LTD 20 CHURCHILL AVE WAGGA WAGGA T: 69263320 F: 69266011 NOMINATED ARCHITECT NOEL THOMSON 5869 ACN: 077 973 623 ABN: 82 077 973 623	
			PROPERTY/ ADDRESS: SHERIDAN LANE	PROJECT TITLE: OLD MILL GUNDAGAI STABILISATION	1901 - A1		
B: BUILDING APPROVAL / QUOTATION	20.07.20	NT	TOWN: GUNDAGAI NSW 2722	SCALE: AS NOTED @ A3	DATE: JAN 2020	DWG NO: A1	REVISION NO: B DRAWN BY: CN
A: BUILDING APPROVAL / QUOTATION	31.01.20	NT					
Revision Notes:		Date: Check:					



Locality Map

DRAWING REGISTER	
Sheet Number	Sheet Name
A1	SITE PLAN - STAGE A
A2	GROUND FLOOR PLAN - STAGE A
A3	LEVEL 1 FLOOR PLAN - STAGE A
A4	LEVEL 2 FLOOR PLAN - STAGE A
A5	SUB-FLOOR / DECK PLAN - STAGE A
A6	ROOF PLAN - EXISTING
A7	ELEVATIONS - STAGE A
A8	ELEVATIONS - STAGE A
A9	SECTIONS - STAGE A

NOTES

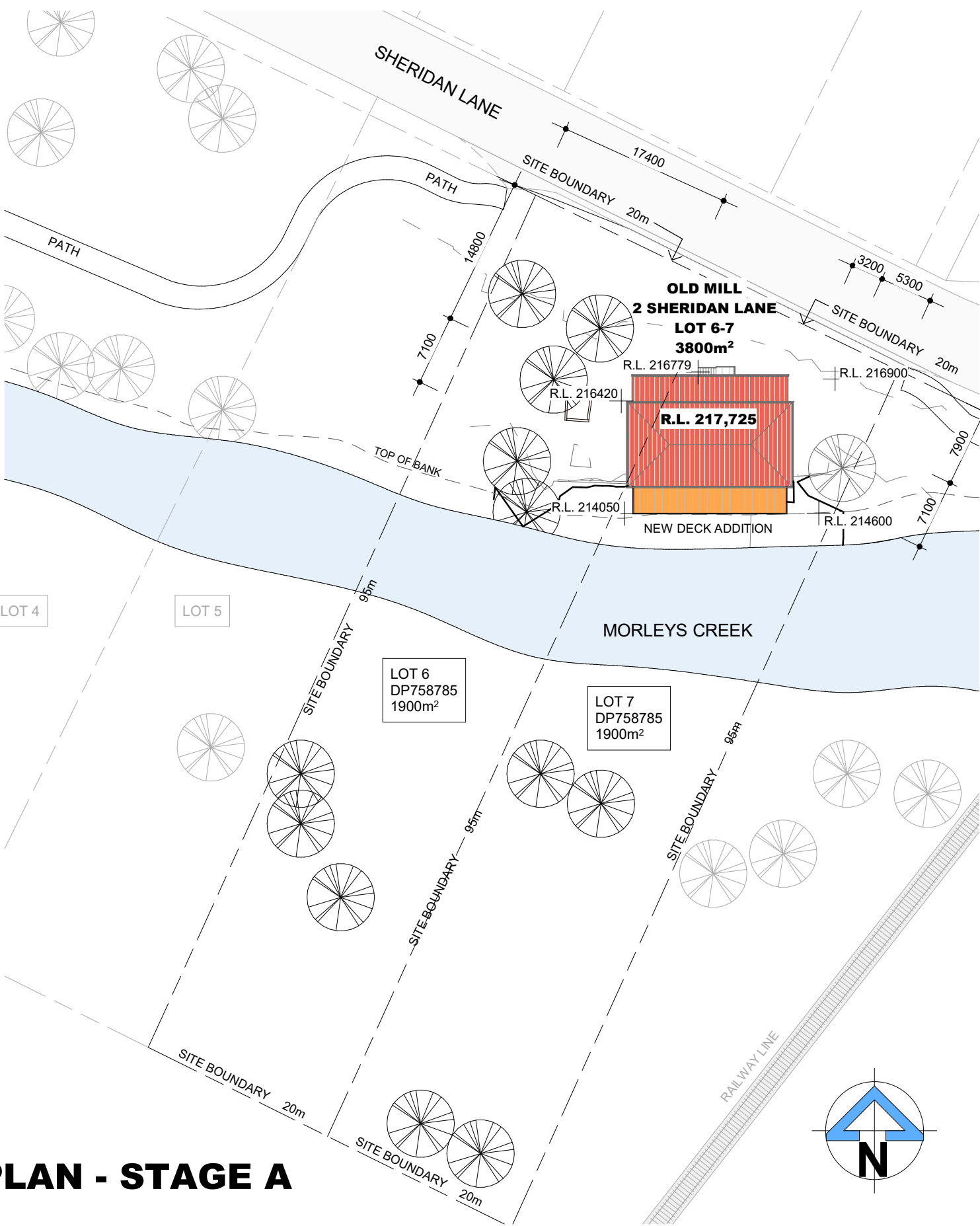
THE CONTRACTOR TO VERIFY ALL DIMENSIONS ON SITE PRIOR TO COMMENCEMENT OF ANY WORK

FIGURED DIMENSIONS SHALL TAKE PRECEDENCE OVER SCALE

PLANS TO BE READ IN CONJUNCTION WITH ALL OTHER DRAWINGS & SPECIFICATIONS

ALL WORK TO COMPLY WITH RELEVANT CODES AND THE BCA

ANY DISCREPANCIES TO BE REPORTED TO PRIMARY CONSULTANT PRIOR TO PROCEEDING WITH WORK



SITE PLAN - STAGE A
1 : 500

			CLIENT: COOTAMUNDRA GUNDAGAI REGIONAL COUNCIL	ARCHITECT: Noel Thomson Architecture Pty Ltd.			PROJECT NO:		NOEL THOMSON ARCHITECTURE PTY LTD 20 CHURCHILL AVE WAGGA WAGGA T: 69263320 F: 69266011 NOMINATED ARCHITECT NOEL THOMSON 5869 ACN: 077 973 623 ABN: 82 077 973 623	
				PROPERTY/ ADDRESS: SHERIDAN LANE	PROJECT TITLE: OLD MILL GUNDAGAI STABILISATION		1901 - A2			
B: BUILDING APPROVAL / QUOTATION	20.07.20	NT	TOWN: GUNDAGAI NSW 2722	SCALE: AS NOTED @ A3	DATE: JAN 2020	DWG NO: A2	REVISION NO: B	DRAWN BY: CN		
A: BUILDING APPROVAL / QUOTATION	31.01.20	NT								
Revision Notes:	Date:	Check:								

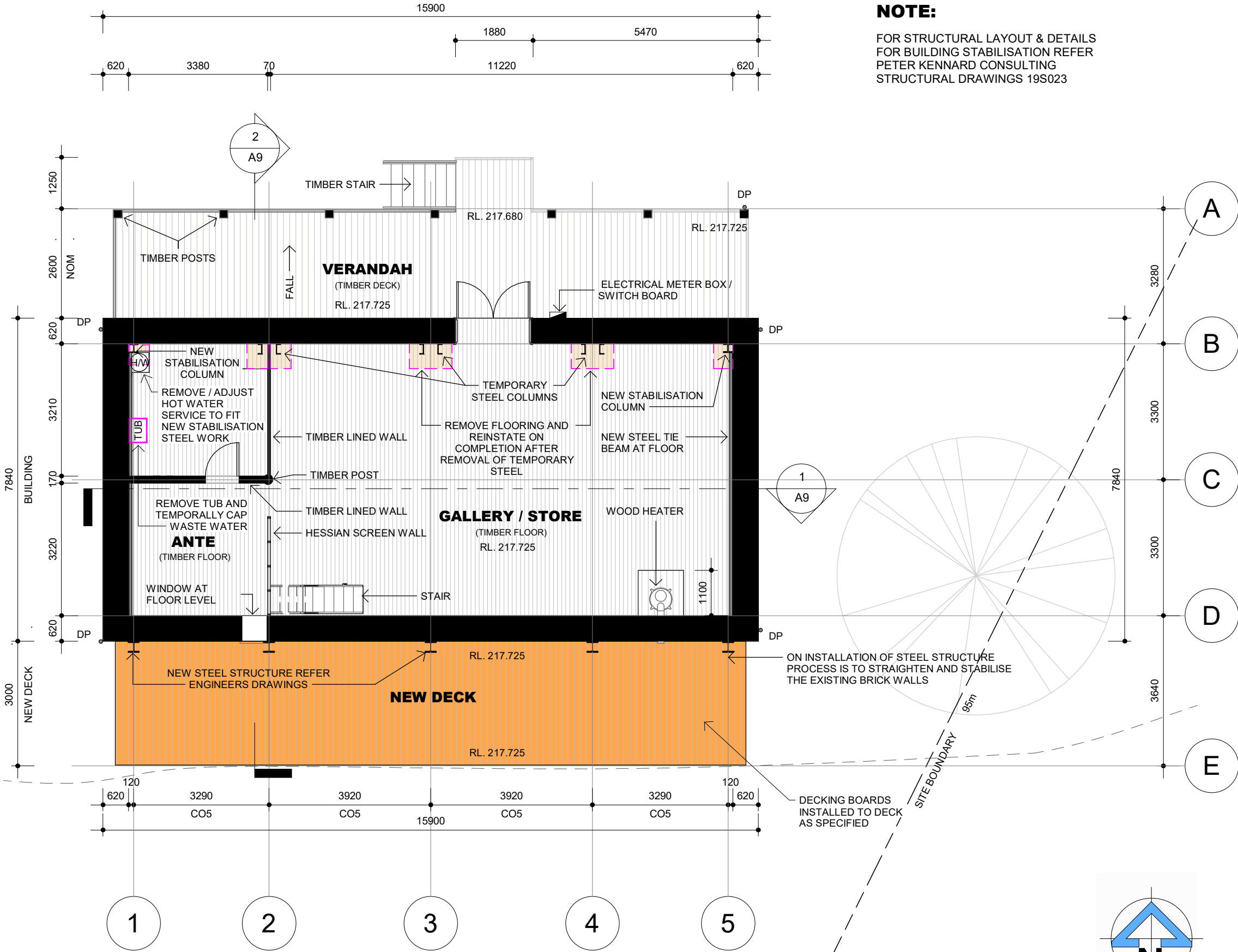
AUSTRALIAN STANDARDS

ALL WORKS TO COMPLY WITH, BUT NOT LIMITED TO THE FOLLOWING AUSTRALIAN STANDARDS

DEMOLITION OF STRUCTURES	AS 2601
MASONRY STRUCTURES	AS 3700
CONCRETE STRUCTURES	AS 3600
STEEL STRUCTURES	AS 4100

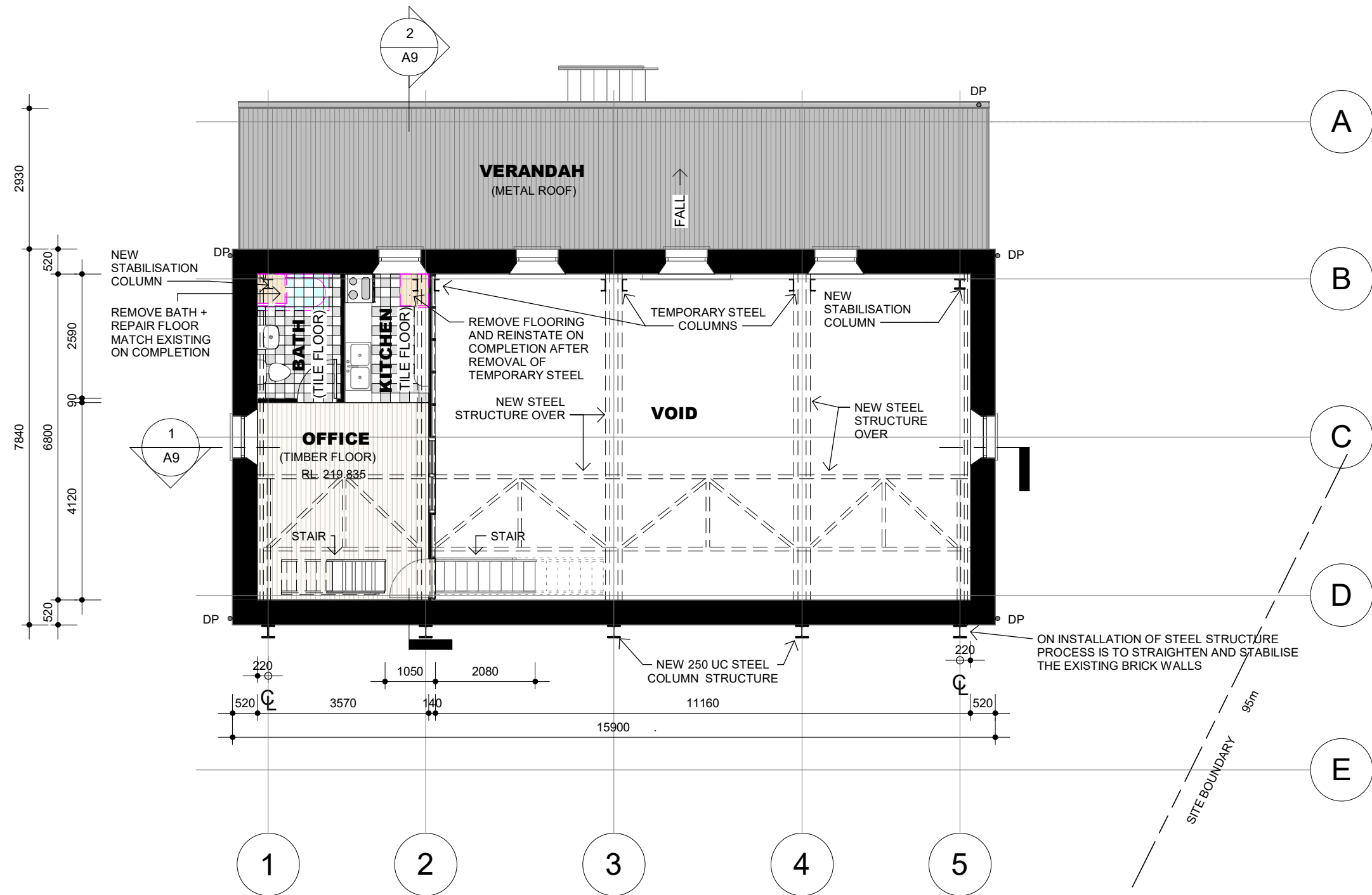
NOTE:

FOR STRUCTURAL LAYOUT & DETAILS FOR BUILDING STABILISATION REFER PETER KENNARD CONSULTING STRUCTURAL DRAWINGS 19S023



GROUND FLOOR PLAN - STAGE A
1: 100

			CLIENT: COOTAMUNDRA GUNDAGAI REGIONAL COUNCIL	ARCHITECT: Noel Thomson Architecture Pty Ltd.			PROJECT NO:		<div>NOEL THOMSON ARCHITECTURE PTY LTD</div> <div>20 CHURCHILL AVE WAGGA WAGGA</div> <div>T: 69263320 F: 69266011</div> <div>NOMINATED ARCHITECT</div> <div>NOEL THOMSON 5869</div> <div>ACN: 077 973 623</div> <div>ABN: 82 077 973 623</div> <div>NOEL THOMSON ARCHITECTURE</div>
				PROPERTY/ ADDRESS: SHERIDAN LANE	PROJECT TITLE: OLD MILL GUNDAGAI STABILISATION			1901 - A3	
B: BUILDING APPROVAL / QUOTATION	20.07.20	NT	TOWN: GUNDAGAI NSW 2722		SCALE: AS NOTED @ A3	DATE: JAN 2020	DWG NO: A3	REVISION NO: B	
A: BUILDING APPROVAL / QUOTATION	31.01.20	NT							
Revision Notes:	Date:	Check:							

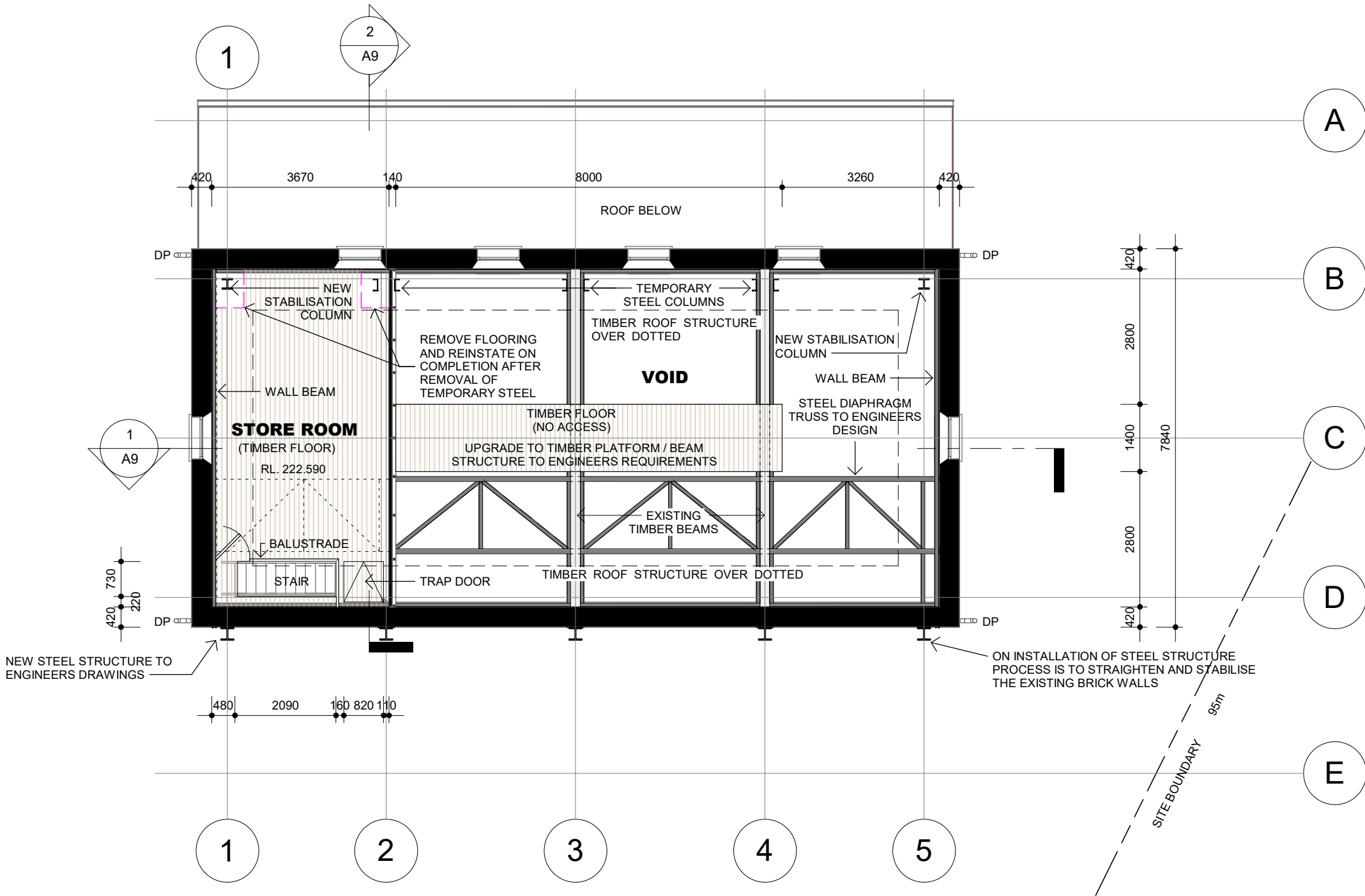


LEVEL 1 FLOOR PLAN - STAGE A
1 : 100

			CLIENT: COOTAMUNDRA GUNDAGAI REGIONAL COUNCIL	ARCHITECT: Noel Thomson Architecture Pty Ltd.			PROJECT NO:		NOEL THOMSON ARCHITECTURE PTY LTD 20 CHURCHILL AVE WAGGA WAGGA T: 69263320 F: 69266011 NOMINATED ARCHITECT NOEL THOMSON 5869 ACN: 077 973 623 ABN: 82 077 973 623	
				PROPERTY/ ADDRESS: SHERIDAN LANE	PROJECT TITLE: OLD MILL GUNDAGAI STABILISATION			1901 - A4		
B: BUILDING APPROVAL / QUOTATION	20.07.20	NT	TOWN: GUNDAGAI NSW 2722		SCALE: AS NOTED @ A3	DATE: JAN 2020	DWG NO: A4	REVISION NO: B	DRAWN BY: CN	
A: BUILDING APPROVAL / QUOTATION	31.01.20	NT								
Revision Notes:	Date:	Check:								

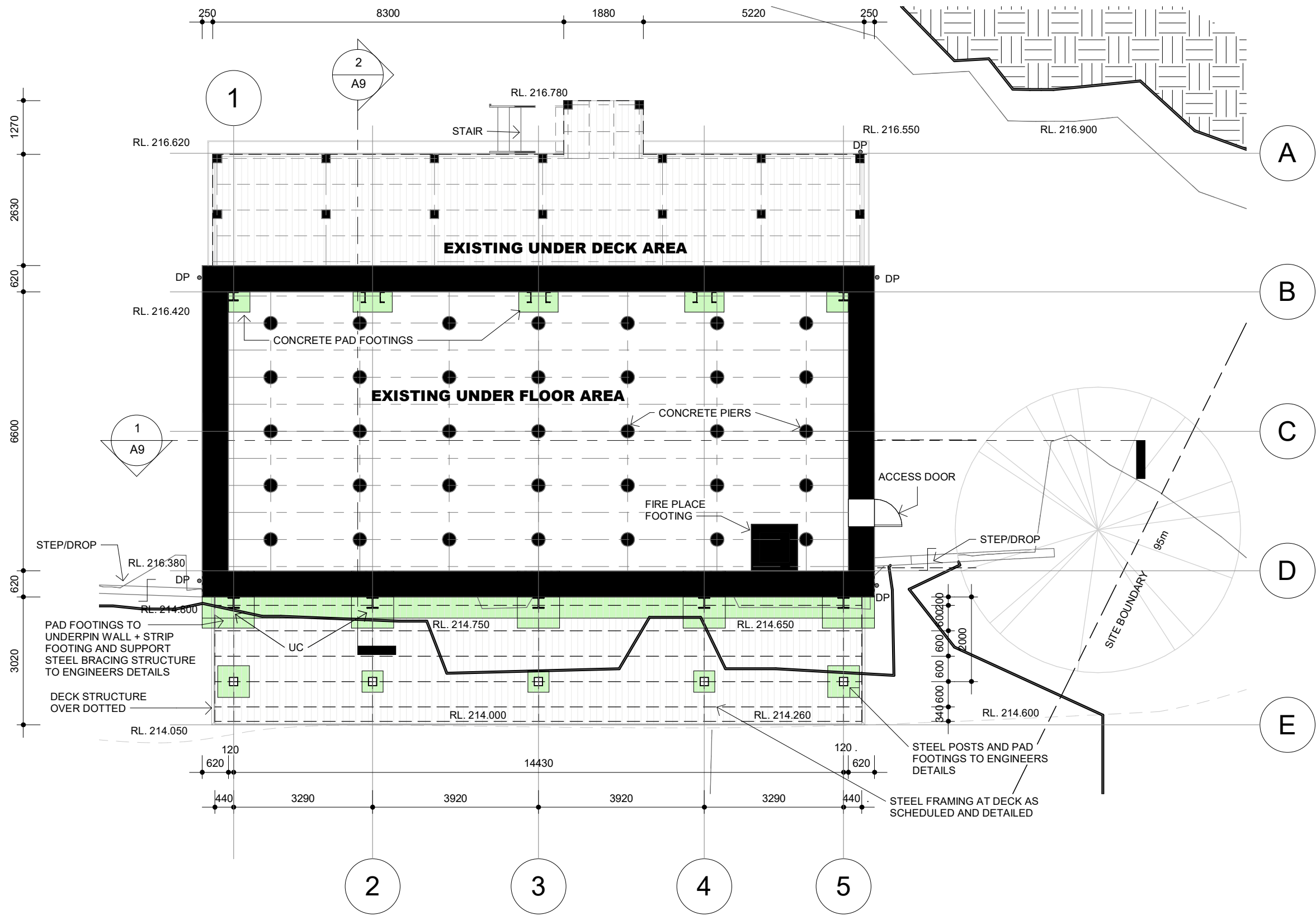


NOTE:
CENTRE OF TIMBER BEAM
EQUALS GRID CENTRELINE +
STEEL STRUCTURE



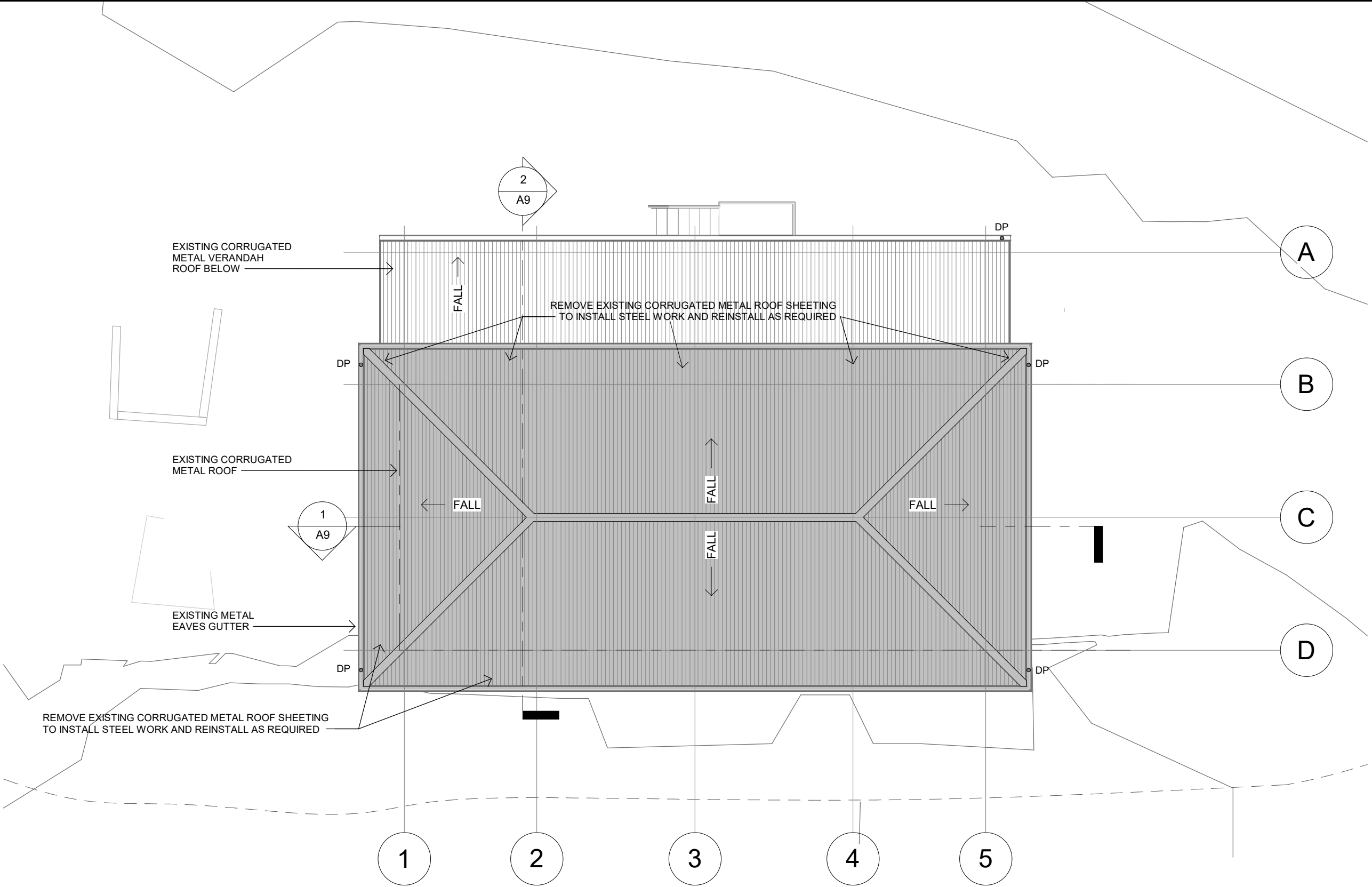
LEVEL 2 FLOOR PLAN - STAGE A
1 : 100

			CLIENT: COOTAMUNDRA GUNDAGAI REGIONAL COUNCIL	ARCHITECT: Noel Thomson Architecture Pty Ltd.			PROJECT NO:		NOEL THOMSON ARCHITECTURE PTY LTD 20 CHURCHILL AVE WAGGA WAGGA T: 69263320 F: 69266011 NOMINATED ARCHITECT NOEL THOMSON 5869 ACN: 077 973 623 ABN: 82 077 973 623	
				PROPERTY/ ADDRESS: SHERIDAN LANE	PROJECT TITLE: OLD MILL GUNDAGAI STABILISATION			1901 - A5		
B: BUILDING APPROVAL / QUOTATION	20.07.20	NT	TOWN: GUNDAGAI NSW 2722		SCALE: AS NOTED @ A3	DATE: JAN 2020	DWG NO: A5	REVISION NO: B	DRAWN BY: CN	
A: BUILDING APPROVAL / QUOTATION	31.01.20	NT								
Revision Notes:	Date:	Check:								



SUB-FLOOR / DECK AREA - STAGE A
1 : 100

			CLIENT: COOTAMUNDRA GUNDAGAI REGIONAL COUNCIL	ARCHITECT: Noel Thomson Architecture Pty Ltd.	PROJECT NO:	NOEL THOMSON ARCHITECTURE PTY LTD 20 CHURCHILL AVE WAGGA WAGGA T: 69263320 F: 69266011	
			PROPERTY/ ADDRESS: SHERIDAN LANE	PROJECT TITLE: OLD MILL GUNDAGAI STABILISATION	1901 - A6	NOMINATED ARCHITECT NOEL THOMSON 5869 ACN: 077 973 623 ABN: 82 077 973 623	
B: BUILDING APPROVAL / QUOTATION	20.07.20	NT	TOWN: GUNDAGAI NSW 2722	SCALE: AS NOTED @ A3	DATE: JAN 2020	DWG NO: A6	REVISION NO: B
A: BUILDING APPROVAL / QUOTATION	31.01.20	NT					DRAWN BY: CN
Revision Notes:		Date:	Check:				

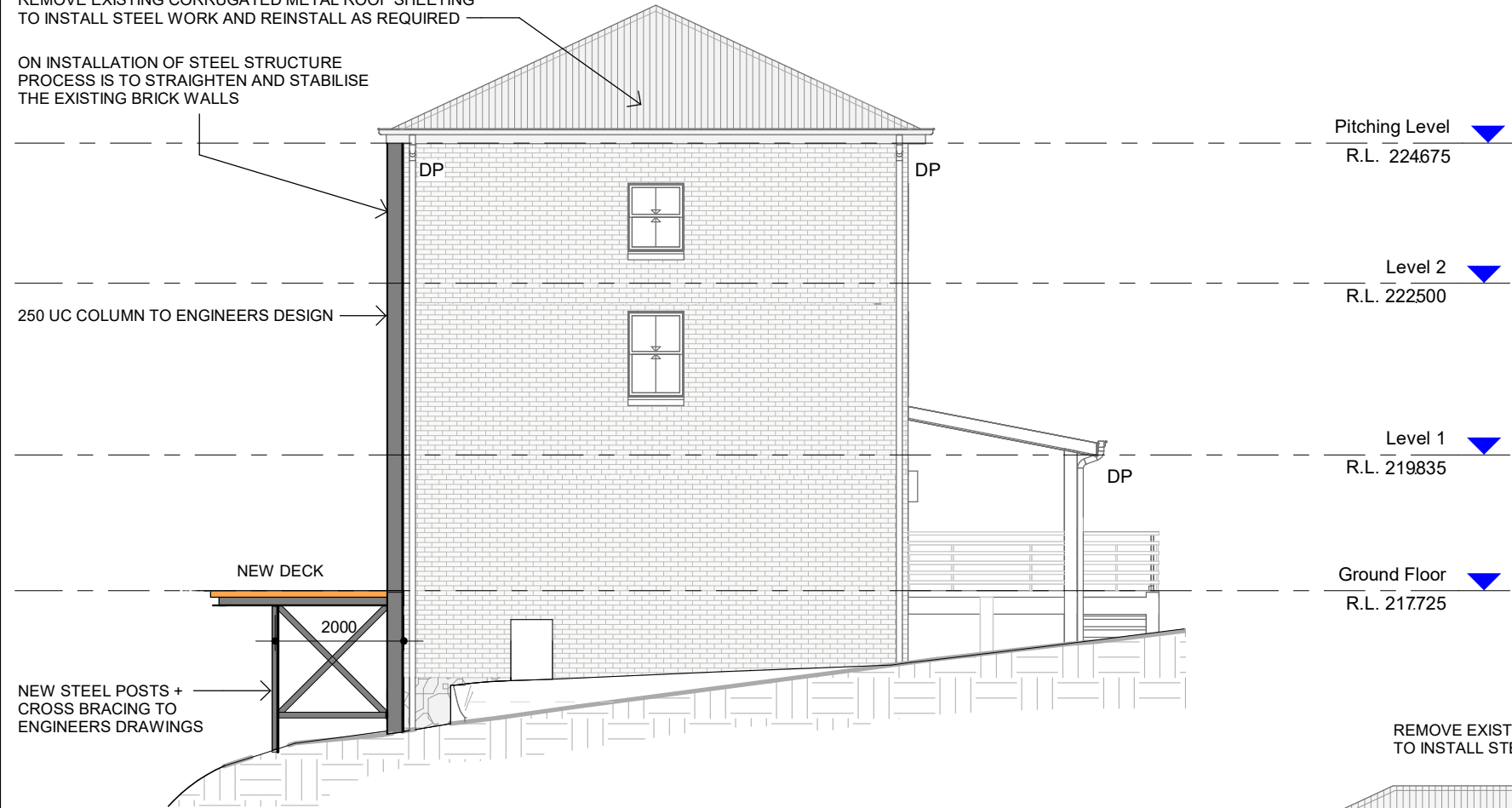


ROOF PLAN - EXISTING
1 : 100

			CLIENT: COOTAMUNDRA GUNDAGAI REGIONAL COUNCIL	ARCHITECT: Noel Thomson Architecture Pty Ltd.	PROJECT NO:	1901 - A7		NOEL THOMSON ARCHITECTURE PTY LTD 20 CHURCHILL AVE WAGGA WAGGA T: 69263320 F: 69266011 NOMINATED ARCHITECT NOEL THOMSON 5869 ACN: 077 973 623 ABN: 82 077 973 623	
			PROPERTY/ ADDRESS: SHERIDAN LANE	PROJECT TITLE: OLD MILL GUNDAGAI STABILISATION					
B: BUILDING APPROVAL / QUOTATION	20.07.20	NT	TOWN: GUNDAGAI NSW 2722	SCALE: AS NOTED @ A3	DATE: JAN 2020	DWG NO: A7	REVISION NO: B	DRAWN BY: CN	
A: BUILDING APPROVAL / QUOTATION	31.01.20	NT							
Revision Notes:		Date:	Check:						

REMOVE EXISTING CORRUGATED METAL ROOF SHEETING TO INSTALL STEEL WORK AND REINSTALL AS REQUIRED

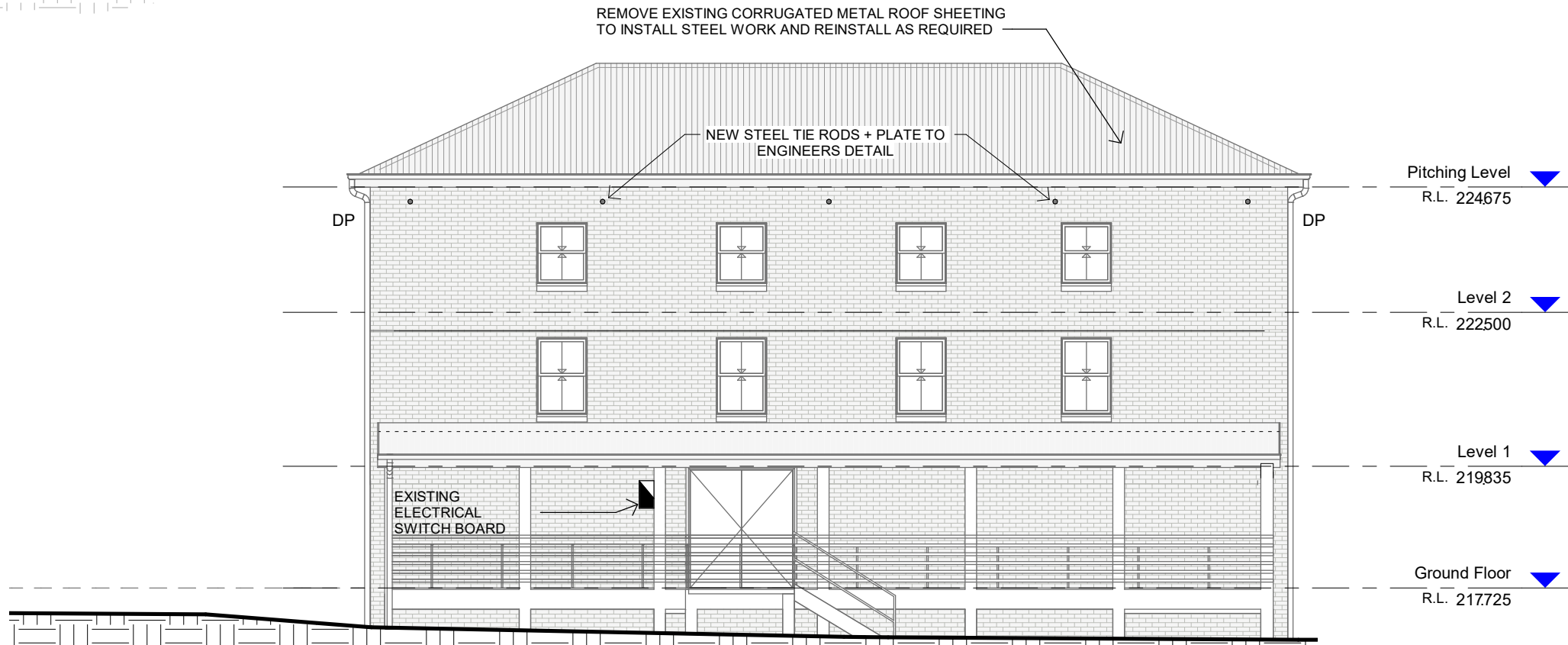
ON INSTALLATION OF STEEL STRUCTURE PROCESS IS TO STRAIGHTEN AND STABILISE THE EXISTING BRICK WALLS



EAST ELEVATION - STAGE
1 : 100

NOTE:
UNDERTAKE REPAIRS TO BRICKWORK WHERE CRACKING OCCURS IN ACCORDANCE WITH ENGINEERS REQUIREMENTS

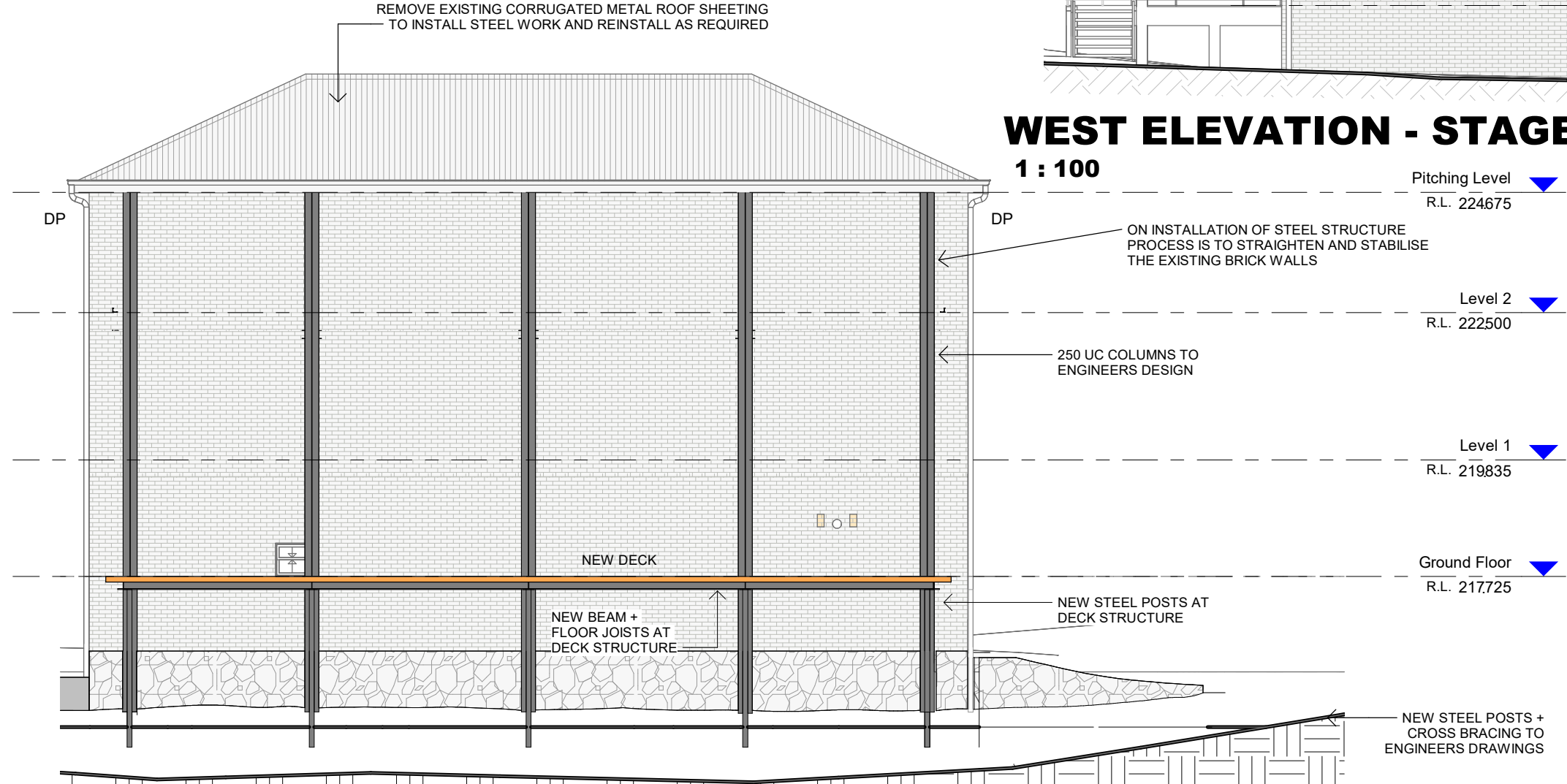
REMOVE EXISTING CORRUGATED METAL ROOF SHEETING TO INSTALL STEEL WORK AND REINSTALL AS REQUIRED



NORTH ELEVATION - STAGE A
1 : 100


			CLIENT: COOTAMUNDRA GUNDAGAI REGIONAL COUNCIL	ARCHITECT: Noel Thomson Architecture Pty Ltd.			PROJECT NO: 1901 - A8		<div>NOEL THOMSON ARCHITECTURE PTY LTD</div> <div>20 CHURCHILL AVE WAGGA WAGGA</div> <div>T: 69263320 F: 69266011</div> <div>NOMINATED ARCHITECT</div> <div>NOEL THOMSON 5869</div> <div>ACN: 077 973 623</div> <div>ABN: 82 077 973 623</div> <div>NOEL THOMSON ARCHITECTURE</div>
				PROPERTY/ ADDRESS: SHERIDAN LANE	PROJECT TITLE: OLD MILL GUNDAGAI STABILISATION				
B: BUILDING APPROVAL / QUOTATION	20.07.20	NT	TOWN: GUNDAGAI NSW 2722		SCALE: AS NOTED @ A3	DATE: JAN 2020	DWG NO: A8	REVISION NO: B	
A: BUILDING APPROVAL / QUOTATION	31.01.20	NT							
Revision Notes:	Date:	Check:							

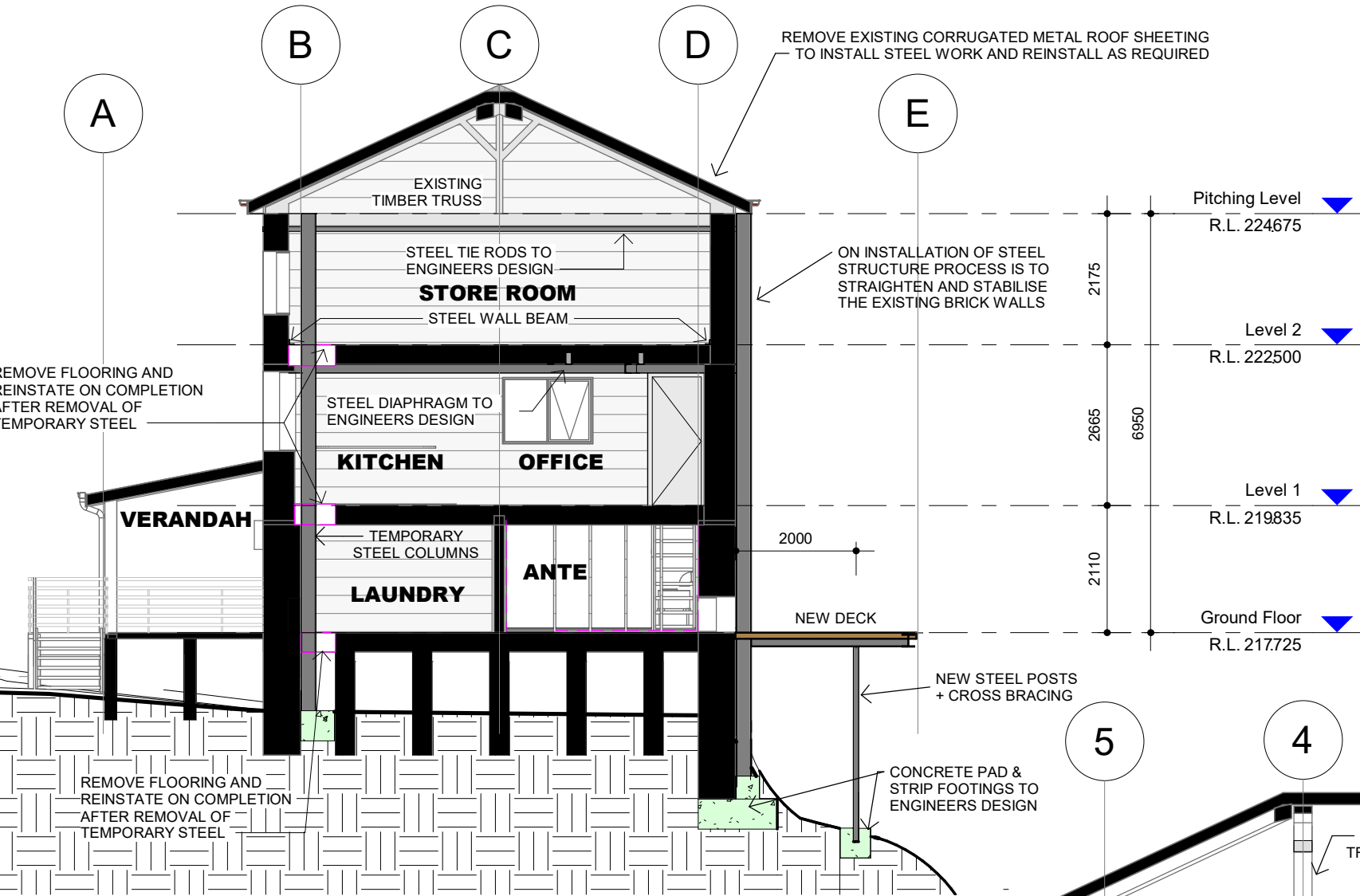
NOTE:
UNDERTAKE REPAIRS TO BRICKWORK WHERE CRACKING OCCURS IN ACCORDANCE WITH ENGINEERS REQUIREMENTS



WEST ELEVATION - STAGE A
1 : 100

SOUTH ELEVATION - STAGE A
1 : 100

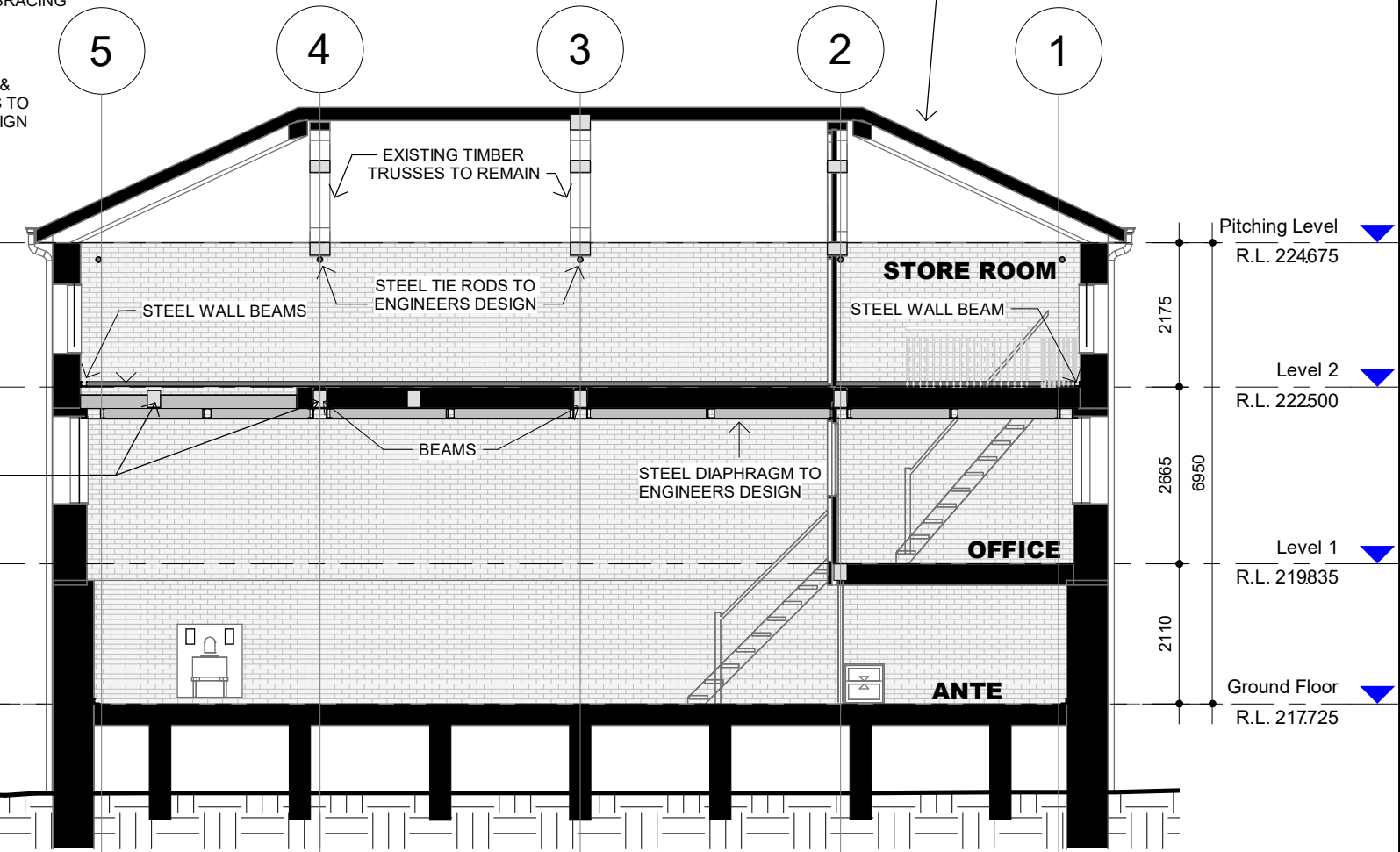
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				PROPERTY/ ADDRESS: SHERIDAN LANE	PROJECT TITLE: OLD MILL GUNDAGAI STABILISATION			1901 - A9		
B: BUILDING APPROVAL / QUOTATION	20.07.20	NT	TOWN: GUNDAGAI NSW 2722		SCALE: AS NOTED @ A3	DATE: JAN 2020	DWG NO: A9	REVISION NO: A	DRAWN BY: CN	
A: BUILDING APPROVAL / QUOTATION	31.01.20	NT								
Revision Notes:	Date:	Check:								



SECTION 2 - STAGE A
1 : 100

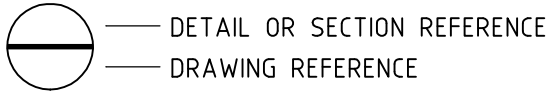
NOTE:
UNDERTAKE REPAIRS TO BRICKWORK
WHERE CRACKING OCCURS

UPGRADE STRUCTURE TO
ENGINEERS REQUIREMENTS



SECTION 1 - STAGE A
1 : 100

GENERAL NOTES

- G1. THESE STRUCTURAL DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL ARCHITECTURAL AND OTHER CONSULTANTS' DRAWINGS AND SPECIFICATIONS AND WITH SUCH OTHER WRITTEN INSTRUCTIONS AS MAY BE ISSUED DURING THE COURSE OF THE CONTRACT. ANY DISCREPANCY SHALL BE REFERRED TO THE STRUCTURAL ENGINEER BEFORE PROCEEDING WITH THE WORK.
- G2. DO NOT COMMENCE CONSTRUCTION USING THESE STRUCTURAL DRAWINGS UNTIL A CONSTRUCTION CERTIFICATE IS ISSUED BY THE PRINCIPAL CERTIFYING AUTHORITY.
- G3. ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE RELEVANT CURRENT STANDARDS AUSTRALIA CODES AND WITH THE BUILDING CODE OF AUSTRALIA.
- G4. ALL SET OUT DIMENSIONS SHOWN ON THESE STRUCTURAL DRAWINGS SHALL BE VERIFIED BY THE BUILDER ON SITE. DO NOT SCALE THESE STRUCTURAL DRAWINGS FOR DIMENSIONS.
- G5. UNLESS NOTED OTHERWISE ALL LEVELS ARE IN METRES AND ALL DIMENSIONS ARE IN MILLIMETRES.
- G6. THE METHOD OF CONSTRUCTION AND THE MAINTENANCE OF SAFETY DURING CONSTRUCTION ARE THE RESPONSIBILITY OF THE BUILDER. IF ANY STRUCTURAL ELEMENT PRESENTS DIFFICULTY IN RESPECT OF CONSTRUCTABILITY OR SAFETY, THE MATTER SHALL BE REFERRED TO THE STRUCTURAL ENGINEER FOR RESOLUTION BEFORE PROCEEDING WITH THE WORK.
- G7. DURING CONSTRUCTION THE STRUCTURE SHALL BE MAINTAINED IN A STABLE CONDITION AND NO PART SHALL BE OVERLOADED. THE BUILDER SHALL PROVIDE TEMPORARY BRACING, SHORING AND PROPPING IN ORDER TO KEEP THE BUILDING WORKS AND EXCAVATIONS STABLE AT ALL TIMES.
- G8. THE BUILDER IS RESPONSIBLE FOR THE ADEQUACY OF ALL TEMPORARY WORKS INCLUDING SHORING, PROPPING AND BRACING AND WHERE NECESSARY IS TO ENGAGE A STRUCTURAL ENGINEER TO DESIGN AND CERTIFY HIS TEMPORARY WORKS.
- G9. IF THERE IS A DISCREPANCY IN MEMBER SIZES FOR ANY COMPONENT, ASSUME FOR PRICING PURPOSES ONLY THAT THE LARGER OR MORE EXPENSIVE SIZE IS CORRECT. REFER TO STRUCTURAL ENGINEER FOR DESIGN BEFORE DETAILING OR CONSTRUCTION.
- G10. DETAIL AND SECTION IDENTIFICATION
- 

— DETAIL OR SECTION REFERENCE
— DRAWING REFERENCE
- G11. THE RLS, SHOWN IN THESE DRAWINGS ARE APPROXIMATE AND ARE FOR THE SOLE PURPOSE OF ASSISTING THE STRUCTURAL DOCUMENTATION. THEY MUST NOT BE USED FOR CONSTRUCTION. REFER TO THE ARCHITECTS DRAWINGS FOR ALL CONSTRUCTION RLS.

STRUCTURAL DESIGN LOADINGS

- L1. THE STRUCTURAL COMPONENTS DETAILED ON THESE STRUCTURAL DRAWINGS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE RELEVANT STANDARDS AUSTRALIA CODES AND THE BUILDING CODE OF AUSTRALIA FOR THE FOLLOWING LOADINGS. REFER TO ARCHITECTURAL DRAWINGS FOR PROPOSED FLOOR USAGE.
- L2. SUPERIMPOSED LOADS
- | FLOOR USAGE | LIVE LOAD | | SUPERIMPOSED DEAD LOAD (kPa) |
|------------------|-----------|-------------|------------------------------|
| | UDL (kPa) | POINT (kPa) | |
| ROOF | 0.25 | | |
| GROUND FLOOR | 3.0 | | |
| EXISTING LEVEL 2 | 2.5 | | |
| DECKS & RAMPS | 5.0 | | |
- L3. WIND LOADS IN ACCORDANCE WITH AS1170.2
- | REGION | |
|--|------------------|
| STRUCTURAL IMPORTANCE LEVEL AS DEFINED IN BCA - PART B1 | 2 |
| REGIONAL WIND SPEED
Vr (Ultimate)
Vr (Servicability) | 45 m/s
37 m/s |
| TERRAIN CATEGORY | 2.5 |
| TERRAIN/HEIGHT MULTIPLIER Mz,cat | 0.87 |
| SHIELDING MULTIPLIER Ms | 1.0 |
| TOPOGRAPHIC MULTIPLIER Mt | 1.0 |
| HILL-SHAPE MULTIPLIER Mh | 1.0 |
- L4. EARTHQUAKE DESIGN PARAMETERS TO AS1170.4
- | | |
|---|------|
| STRUCTURAL IMPORTANCE LEVEL AS DEFINED IN BCA - PART B1 | 3 |
| PROBABILITY FACTOR kp | 0.6 |
| HAZARD FACTOR Z | 0.09 |
| SITE SUB-SOIL CLASS | Ce |
| EARTHQUAKE DESIGN CATEGORY | II |

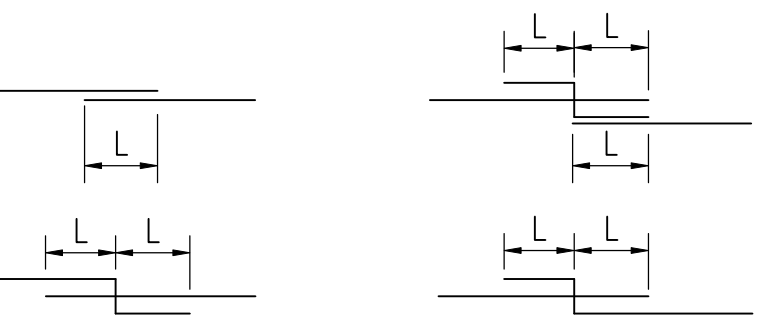
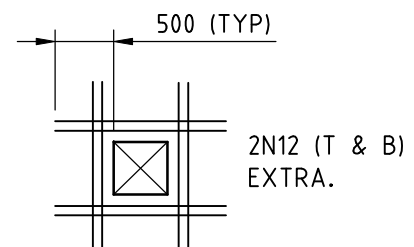
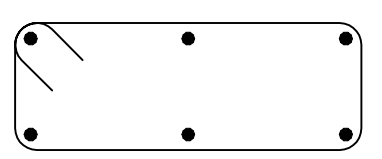
CONCRETE

- C1. ALL WORKMANSHIP AND MATERIALS SHALL COMPLY WITH AS3600 EXCEPT WHERE VARIED BY THE CONTRACT DOCUMENTS
- C2. CONCRETE QUALITY
- * C2.1 ALL CONCRETE SHALL COMPLY WITH AS1379.
 - * C2.2 NO BRECCIA TYPE AGGREGATE IS TO BE USED
 - * C2.3 COMPRESSIVE STRENGTH GRADES
- | ELEMENT | STRENGTH GRADE (MPa) | CEMENT TYPE TO AS3972 | SLUMP (mm) | MAXIMUM AGGREGATE SIZE (mm) |
|----------|----------------------|-----------------------|------------|-----------------------------|
| FOOTINGS | 25 | A | 80 | 20 |
- SPECIAL CLASS CONCRETES (PREFIXED S IN THE TABLE) SHALL HAVE THE PROPERTIES OF NORMAL CLASS CONCRETE WITH THE FOLLOWING THE FOLLOWING SPECIAL REQUIREMENTS:
- CLASS S - SHRINKAGE STRAIN SHALL NOT EXCEEDX 10-6 AT 56 DAYS IN ACCORDANCE WITH AS1012
- CEMENT SHALL BE TYPE SL TO AS 3972
- C3. CONCRETE PROFILES
- * C3.1 SIZES OF CONCRETE ELEMENTS DO NOT INCLUDE THICKNESS OF APPLIED FINISHES.
 - * C3.2 BEAM DEPTHS ARE WRITTEN FIRST AND INCLUDE THE SLAB THICKNESS.
 - * C3.3 NO HOLES, CHASES, OR EMBEDMENT OF PIPES OTHER THAN SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE MADE IN CONCRETE MEMBERS WITHOUT THE PRIOR WRITTEN APPROVAL OF THE ENGINEER.
 - * C3.4 CANTILEVERS - PROVIDE UPWARD CAMBER IN FORMWORK FOR REINFORCED CONCRETE CANTILEVERS OF L/120, WHERE L IS THE PROJECTION BEYOND THE COLUMN OR WALL FACE. MAINTAIN THE SLAB AND BEAM DEPTHS SHOWN.
 - * C3.5 PROVIDE DRIP GROOVES AT ALL EXPOSED EDGES. CHAMFERS, DRIP GROOVES, REGLETS, ETC TO BE TO ARCHITECT'S DETAILS. MAINTAIN COVER TO REINFORCEMENT AT THESE DETAILS.
 - * C3.6 CONSTRUCTION JOINTS NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE TO THE WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER.
 - * C3.7 CONDUITS, PIPES ETC. SHALL ONLY BE LOCATED IN THE MIDDLE ONE THIRD OF SLAB DEPTH AND SPACED AT NOT LESS THAN 3 DIAMETERS. DO NOT PLACE PIPES OR CONDUITS WITHIN THE COVER TO THE REINFORCEMENT.
- C4. COVER TO REINFORCEMENT
- | CONDITION | MINIMUM COVER |
|--|---------------|
| SURFACES IN CONTACT WITH GROUND: WITHOUT MEMBRANE WITH MEMBRANE: | 50mm |
| * SLABS | 30mm |
| * FOOTINGS | 50mm |
| SURFACES ABOVE GROUND -EXPOSED | 30mm |
- C5. THE FINISHED CONCRETE SHALL BE A DENSE HOMOGENEOUS MASS, COMPLETELY FILLING THE FORMWORK, THOROUGHLY EMBEDDING THE REINFORCEMENT AND FREE OF STONE POCKETS. COMPACT ALL CONCRETE WITH MECHANICAL VIBRATORS, INCLUDING FOOTINGS AND SLABS ON GROUND.
- C6. CURING OF CONCRETE
- CURE ALL CONCRETE AS FOLLOWS:
- KEEP SURFACES CONTINUOUSLY WET FOR 3 DAYS, FOLLOWED BY
 - PREVENT MOISTURE LOSS FOR THE NEXT 4 DAYS, USING POLYTHENE SHEETING OR WET HESSIAN PROTECTED FROM WIND AND TRAFFIC, AND THEN
 - ALLOW GRADUAL DRYING OUT
- CURING COMPOUNDS MAY BE USED, PROVIDED THAT THEY COMPLY WITH AS3799. AND DO NOT AFFECT FLOOR FINISHES. PVA BASED CURING COMPOUNDS ARE NOT ACCEPTABLE.
- C7. SLIP JOINTS TO BE USED ON ALL LOAD-BEARING MASONRY WALLS. USE 2 LAYERS OF GALVANISED FLAT STEEL WITH GRAPHITE GREASE BETWEEN.
- C8. SLAB REINFORCEMENT AT SUPPORTING WALLS
- SLAB BARS SHALL EXTEND 70MM ONTO SUPPORTING WALLS, WITH 50% OF BOTTOM BARS COGGED TO ACHIEVE ANCHORAGE AT SIMPLY SUPPORTED ENDS.
- MESH IN SLABS SHALL EXTEND 70MM ONTO SUPPORTING WALLS WITH A CROSS WIRE.
- C9. MESH LAPPED SPLICES
- LAPS IN MESH (FABRIC) SHALL COMPLY WITH AS3600. THE TWO OUTERMOST TRANSVERSE WIRES OF ONE SHEET SHALL OVERLAP THE TWO OUTERMOST TRANSVERSE WIRES OF THE SHEET BEING LAPPED.

REINFORCEMENT FOR CONCRETE

- R1. REINFORCEMENT QUALITY AND NOTATION
- * R1.1 BAR REINFORCEMENT
- | SYMBOL | BAR SHAPE | STRENGTH GRADE (MPa) | DUCTILITY CLASS | TO COMPLY WITH AUST. STANDARD |
|--------|------------------------------------|----------------------|-----------------|-------------------------------|
| N | DEFORMED RIBBED BAR | 500 | NORMAL | AS 4671 |
| R | PLAIN ROUND BAR | 250 | NORMAL | AS 4671 |
| *Y | DEFORMED BAR *SEE NOTE -SUPERSEDED | 400 | NORMAL | AS 1302 |
- ALL REINFORCING BARS SHALL BE GRADE D500N TO AS4671 UNO. REINFORCEMENT NOTATION IS AS FOLLOWS:
- NUMBER OF BARS IN GROUP, BAR GRADE, NOMINAL BAR SIZE IN mm, SPACING IN mm
- E.G. 17 N16-250, WHERE N16 DENOTES A DEFORMED RIBBED BAR, OF GRADE 500MPa NORMAL DUCTILITY STEEL, WITH A NOMINAL 16mm DIAMETER, AT 250 SPACING.
- NOTE: Y BARS MAY BE REPLACED WITH N BARS OF SAME SIZE, I.E. DEFORMED RIBBED BAR OF GRADE 500, NORMAL DUCTILITY STEEL.
- * R1.2 Mesh reinforcement
- | SYMBOL | BAR SHAPE | STRENGTH GRADE (MPa) | DUCTILITY CLASS | TO COMPLY WITH AUST. STANDARD |
|--------|--|----------------------|-----------------|-------------------------------|
| RL | RECTANGULAR MESH OF DEFORMED RIBBED BARS | 500 | Low | AS 4671 |
| SL | SQUARE MESH OF DEFORMED RIBBED BARS | 500 | Low | AS 4671 |
| L12TM | TRENCH MESH | 500 | Low | AS 4671 |
- ALL MESH SHALL BE GRADE 500L TO AS4671 UNO. THE NUMBERS FOLLOWING THE SYMBOL DENOTE THE PRODUCT CODE. FOR EXAMPLE, SL92 DENOTES A SQUARE MESH OF 9mm (NOMINAL DIAMETER) DEFORMED RIBBED BARS AT 200mm CENTRES, OF GRADE 500MPa LOW DUCTILITY STEEL.
- R2. COVER TO REINFORCEMENT
- COVER TO REINFORCEMENT FOR DURABILITY SHALL BE AS FOLLOWS UNO.
- | CONDITION | COVER (mm) |
|--|------------|
| SURFACES IN CONTACT WITH GROUND: WITHOUT MEMBRANE WITH MEMBRANE: | 50 mm |
| * SLABS | 30 mm |
| * FOOTINGS | 50 mm |
| SURFACES ABOVE GROUND -INTERIOR | |
| • SLABS & BEAMS | 30 mm |
| • COLUMNS | 40 mm |
| SURFACES ABOVE GROUND -EXPOSED | |
| • SLABS & BEAMS | 30 mm |
| • COLUMNS | 40 mm |
- COVER SHALL NOT BE LESS THAN THE SIZE OF THE AGGREGATE OR THE MAIN BARS.
- PIPES OR CONDUITS SHALL NOT BE PLACED WITHIN THE COVER TO REINFORCEMENT.
- COVER MAY NEED TO BE INCREASED TO SUIT FIRE RATING -SEE DRAWINGS.
- SUPPORT REINFORCEMENT ON MILD STEEL PLASTIC TIPPED CHAIRS, PLASTIC CHAIRS OR CONCRETE CHAIRS AT NOT GREATER THAN 1 METRE CENTRES BOTH WAYS.
- IN EXPOSURE CONDITION B2 OR C (TO AS3600) USE ONLY PLASTIC OR CONCRETE CHAIRS.
- TIE BARS AT ALTERNATE INTERSECTIONS.
- R3. REINFORCEMENT REPRESENTATION
- REINFORCEMENT IS REPRESENTED DIAGRAMMATICALLY, AND NOT NECESSARILY IN TRUE PROJECTION.
- BARS SHOWN ARE INDICATIVE ONLY AND LENGTHS MAY VARY. BEAM ELEVATIONS TAKE PRECEDENCE OVER SECTIONS. SLAB PLANS TAKE PRECEDENCE OVER SECTIONS. REFER TO SECTIONS FOR EXTRA BARS THAT MAY BE REQUIRED.
- R4. REINFORCEMENT LAYERS:
- B1 DENOTES BOTTOM BARS LAID 1st
- B2 DENOTES BOTTOM BARS LAID 2nd
- T1 DENOTES TOP BARS LAID 1st
- T2 DENOTES TOP BARS LAID 2nd
- R5. DISTRIBUTION REINFORCEMENT
- PROVIDE DISTRIBUTION REINFORCEMENT OR TIE BARS IF NOT SHOWN. USE N12 AT 400, SPLICE 400mm WHERE NECESSARY, AND LAP 400mm WITH MAIN BARS
- R6. REINFORCEMENT LAPPED SPLICES:
- SPLICES IN REINFORCEMENT SHALL BE MADE ONLY IN POSITIONS SHOWN ON THE STRUCTURAL DRAWINGS OR AS OTHERWISE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER.
- LAPS SHALL BE IN ACCORDANCE WITH AS3600 AND NOT LESS THAN THE DEVELOPMENT LENGTH FOR EACH BAR.

REINFORCEMENT FOR CONCRETE (Cont'd)

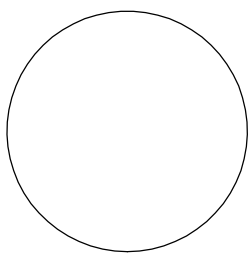
- SLAB AND BEAM REINFORCEMENT
- LAP SPLICES IN SLAB AND BEAM REINFORCEMENT SHALL COMPLY WITH THE TABLE BELOW:
- UNLESS SHOWN OTHERWISE ON THE DRAWINGS, OR
 - UNLESS CALCULATED IN ACCORDANCE WITH AS3600, AND APPROVED IN WRITING BY THE STRUCTURAL ENGINEER.
- FULL STRENGTH LAPS FOR SLAB AND BEAM BARS
- | BAR DIA. | LENGTH L (mm) -SEE DIAGRAMS BELOW | |
|----------|---|---|
| | Bar with 300mm or less depth of conc. below the bar | Bar with more than 300mm depth of conc. below the bar |
| N10 | 500 | 700 |
| N12 | 650 | 800 |
| N16 | 900 | 1100 |
- LAP LENGTHS ARE TO BE INCREASED BY 30% WHEN SLIP FORMS ARE USED
- BARS IN SLABS MAY BE IN STOCK LENGTHS WITH FULL STRENGTH STAGGERED LAPS. (SUBJECT TO APPROVAL FROM THE STRUCTURAL ENGINEER)
- DIAGRAMS:
- 
- COLUMNS - COMPRESSION LAPS
- UNLESS SHOWN OTHERWISE ON THE STRUCTURAL DRAWINGS:
- | BAR DIA. | LAP LENGTH (mm) |
|----------|-----------------|
| N12 | 500 |
| N16 | 650 |
| N20 | 800 |
| N24 | 1000 |
- MESH LAPPED SPLICES
- LAPS IN MESH (FABRIC) SHALL COMPLY WITH AS3600. THE TWO OUTERMOST TRANSVERSE WIRES OF ONE SHEET SHALL OVERLAP THE TWO OUTERMOST TRANSVERSE WIRES OF THE SHEET BEING LAPPED.
- R7. SLAB REINFORCEMENT AT SUPPORTING WALLS
- SLAB BARS SHALL EXTEND 70mm ONTO SUPPORTING WALLS, WITH 50% OF BOTTOM BARS COGGED TO ACHIEVE ANCHORAGE AT SIMPLY SUPPORTED ENDS.
- MESH IN SLABS SHALL EXTEND 70mm ONTO SUPPORTING WALLS WITH A CROSS WIRE.
- R8. CRANK THE REINFORCEMENT IN THE BEAM THAT IS TERMINATING AT AN INTERSECTION UNDER/OVER THE BEAM THAT IS CONTINUING. WHERE BOTH BEAMS TERMINATE, CONSIDER THE SHORTER SPAN AS THE TERMINATING BEAM.
- R9. JOGGLES
- JOGGLES IN BARS TO BE 1 BAR DIAMETER OVER A LENGTH OF 12 BAR DIAMETERS
- R10. WELDING OF REINFORCEMENT
- DO NOT WELD REINFORCEMENT (UNLESS SHOWN ON THE STRUCTURAL DRAWINGS) WITHOUT WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER. APPROVAL FOR WELDING WILL DEPEND ON THE TYPE OF BAR AND ITS LOCATION.
- R11. SITE BENDING OF REINFORCEMENT
- SITE BENDING OF DEFORMED BARS (N OR Y) SHALL BE DONE WITHOUT HEATING, USING MECHANICAL BENDING TOOLS AND A MANDREL OR FORMER WITH A DIAMETER OF 5 TIMES THE BAR SIZE.
- HEATING OF N OR Y BARS REDUCES THEIR STRENGTH.
- R12. TRIMMER BARS
- PENETRATIONS (DENOTED T.P. ON PLAN).
- (FOR PENETRATIONS UP TO 600X600 U.N.O.)
- 
- R13. INSPECTION BY STRUCTURAL ENGINEER
- GIVE AT LEAST 24 HOURS NOTICE TO THE STRUCTURAL ENGINEER FOR INSPECTION OF REINFORCEMENT.
- DO NOT HAVE CONCRETE DELIVERED UNTIL FINAL APPROVAL IS OBTAINED FROM THE STRUCTURAL ENGINEER.
- R14. ALL BEAM TIES ARE TO HAVE BAR ANCHORAGES LOCATED ON THE TOP FACE OF THE BEAM UNO.
- 

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Peter Kennard Consulting

ABN 37 002 836 599
52 Johnston Street, WAGGA WAGGA NSW 2650
T 02 6921 1877 F 02 6921 7415 pkennard@pkengineers.com.au

Structural Engineers

Project
OLD MILL GALLERY GUNDAGAI STAGE 1 - RECTIFICATION
SHERIDAN LANE
GUNDAGAI

Client
OOTAMUNDRA GUNDAGAI REGIONAL COUNCIL

Architect / Project Manager
NOEL THOMSON ARCHITECTURE

Drawing Title
GENERAL CONSTRUCTION NOTES SHEET 1

Scales
NTS

Drawing No.
19S023-S01

Client Project No.

Sheet
1 of 9

Revision
-

STRUCTURAL STEELWORK

- S1 ALL WORKMANSHIP AND MATERIAL SHALL BE IN ACCORDANCE WITH AS4100 EXCEPT WHEN VARIED BY THE CONTRACT DOCUMENTS. FABRICATION SHALL BE CARRIED OUT IN ACCORDANCE WITH SECTION 14 OF AS4100. ERECTION SHALL BE CARRIED OUT IN ACCORDANCE WITH SECTION 15 OF AS4100.
- S2 UNLESS NOTED OTHERWISE ALL STEEL SHALL BE OF THE FOLLOWING GRADE IN ACCORDANCE WITH THE FOLLOWING AUSTRALIAN STANDARDS

Type of steel	Australian Standard	Grade
Universal beams & columns, parallel flange channels & large angles	AS/NZS 3679.1	300
Welded sections	AS/NZS 3679.2	300
Hot milled plates, flats, floor plates, Small angles and slabs	AS/NZS 3678	250
Hollow sections - square & rectangular	AS 1163	C350 or C450 according to Section designation
Circular hollow sections	AS 1163	C350 or C250 according to Section designation
Cold formed purlins and girts	AS 1397	G450 Z350

PROVIDE CERTIFICATES OF COMPLIANCE FOR ALL STEELWORK TO THE STRUCTURAL ENGINEER BEFORE ORDERING.

- S3 WELDING
ALL WELDING SHALL COMPLY WITH AS1554.
FILLET WELDS SHALL BE 6mm CONTINUOUS, CATEGORY GP USING E48XX ELECTRODES OR EQUIVALENT, UNLESS NOTED OTHERWISE. BUTT WELDS SHALL BE COMPLETE PENETRATION BUTT WELDS CATEGORY SP TO AS 1554.1
WHERE WELDS ARE NOT OTHERWISE SPECIFIED THEY ARE TO ACHIEVE THE FULL STRENGTH OF THE MEMBERS JOINED.
- S4 WELD TESTING
THE EXTENT OF NON-DESTRUCTIVE WELD EXAMINATION SHALL BE AS NOTED BELOW. RADIOGRAPHIC OR ULTRASONIC EXAMINATION SHALL BE TO AS1554.1, AS2177.1 AND AS2207 AS APPROPRIATE.

Type of weld and Category	Examination Method	Extent (% of total Length of weld type)
Fillet welds, GP+SP	Visual inspection	100%
Butt welds, GP	Visual inspection	100%
Butt welds, SP	Visual inspection Radiographic or Ultrasonic Inspection	100% 10%

- S5 BOLTS SHALL BE M20 UNLESS NOTED OTHERWISE.
BOLTS SHALL BE 8.8/S UNLESS NOTED OTHERWISE.
ALL BOLTS, NUTS AND WASHERS SHALL BE GALVANISED TO AS1214. COLUMN HOLDING DOWN BOLTS, CAST IN PLACE, SHALL BE 4.6/S UNLESS NOTED OTHERWISE.

Column HD bolt	Embed in concrete	Gog	Concrete edge distance minimum
M16 4.6/S	250	50	160
M20 4.6/S	300	75	200
M24 4.6/S	400	100	260

- FOR DRILLED-IN BOLTS SEE S24
- S6 BOLTS DENOTED 4.6/S ARE COMMERCIAL BOLTS OF STRENGTH GRADE 4.6 TO AS1111, SNUG-TIGHT.
- S7 BOLTS DENOTED 8.8/S, 8.8/TF AND 8.8/TB ARE HIGH STRENGTH STRUCTURAL BOLTS OF STRENGTH GRADE 8.8 TO AS 1252.
- 8.8/S DENOTES BOLTS SNUG-TIGHT
 - 8.8/TF AND 8.8/TB DENOTES BOLTS FULLY TENSIONED TO AS 4100
 - 8.8/TF DENOTES FRICTION JOINT
 - 8.8/TB DENOTES BEARING JOINT

- S8 LOAD INDICATOR WASHERS SHALL BE USED UNDER THE BOLT HEAD FOR ALL 8.8/TF AND 8.8/TB BOLTS. PROVIDE A 75mm COLOUR FLASH AT THESE CONNECTIONS.

- S9 BOLT HOLES AND WASHERS - TYPICAL FOR UP TO M24 (UNLESS SHOWN OTHERWISE ON DRAWINGS)

STEELWORK CONT'D

S9.1 TYPICAL CONNECTIONS			
Connection type	Bolt holes shall be round. Size = bolt diameter plus:	Washers	
		Bolt type	Washers - HD Galvanised to AS1214
Steel to steel	2mm	4.6/S	To AS1111 (37 OD x 3mm thick for M20)
		8.8/S	To AS1252 (39 OD x 4mm nominal thick for M20)
		8.8/TF	To AS1252 (39 OD x 4mm nominal thick for M20)
		8.8/TF	Plus load indicator washers under bolt head
Steel to concrete	4mm		Minimum 4mm thick plate washer
Column baseplates	6mm	M20 4.6/S	45x45x4mm plate washer
		M24 4.6/S	50x50x5mm plate washer

- S9.2 CONNECTIONS TO TILT UP CONCRETE WALL PANELS
FOR CONNECTIONS TO CAST IN FERRULES IN TILT UP WALL PANELS, BOLT HOLES SHALL BE 6mm OVERSIZE WIDE X LONG SLOTTED HOLES (UNLESS SHOWN OTHERWISE ON DRAWINGS).

Bolt	Hole size	Washer - to completely cover slotted hole
M20 8.8/S	26 wide x 50 mm	75 x 75 x 8mm plate washer

S9.3 SLOTTED HOLES FOR STEEL TO STEEL CONNECTIONS			
Type	Hole size		Washers
	Width	Length	
Short slotted holes	2mm oversize	Bolt diameter + 10mm	Provide hardened or plate washer under both bolt & nut
Long slotted holes	2mm oversize	2.5 x bolt diameter	Minimum washer thickness 8mm. Washer shall completely cover the long slotted hole. Provide washer under both bolt and nut

- S9.4 HOLES OUT OF POSITION
IF HOLES ARE OUT OF POSITION, ADVISE ENGINEER BEFORE ENLARGING HOLES.

- S10 CONNECTIONS
CONNECTION DETAILS SHOWN ON STRUCTURAL DRAWINGS ARE TYPICAL ONLY. WHERE A DETAIL IS NOT SHOWN THE FABRICATOR/SHOP DETAILER SHALL PREPARE DETAILS IN ACCORDANCE WITH AS4100 AND THE AISC PUBLICATIONS "DESIGN OF STRUCTURAL CONNECTIONS" AND "STANDARDISED STRUCTURAL CONNECTIONS". THESE DETAILS SHALL TAKE DUE ACCOUNT OF ARCHITECTURAL AND SERVICE REQUIREMENTS AND SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL. CONNECTIONS SHALL HAVE AT LEAST TWO BOLTS. THE ENGINEER WILL ADVISE DESIGN LOADS AS REQUIRED. ALL COSTS AND THE IMPLICATIONS ASSOCIATED WITH THESE WORKS ARE TO BE ALLOWED FOR BY THE CONTRACTOR.

- S11 ALL PLATES AND STIFFENERS SHALL BE 10mm THICK UNLESS NOTED OTHERWISE.
- PURLIN CLEATS SHALL BE 8mm THICK UNLESS NOTED OTHERWISE
 - GIRT CLEATS SHALL BE 10mm THICK UNLESS NOTED OTHERWISE

- S12 CO-ORDINATION
THE CONTRACTOR SHALL MAKE THE NECESSARY ALLOWANCES FOR CO-ORDINATING ALL ARCHITECTURAL AND STRUCTURAL ELEMENTS IN THE PREPARATION OF STRUCTURAL STEELWORK SHOP DRAWINGS AND SUBSEQUENT FABRICATION AND ERECTION.

- S13 PROVIDE ALL NECESSARY PURLIN, GIRT AND TRIMMING ELEMENTS AS REQUIRED TO SUPPORT ALL ROOF AND WALL SHEETING/CLADDING EDGES, VALLEYS, HIPS AND PENETRATIONS.

- S14 PURLIN AND GIRTS SHALL BE "LYSAGHTS" OR "STRAMIT" OR APPROVED, INSTALLED IN ACCORDANCE WITH MANUFACTURER'S WRITTEN DIRECTIONS. USE WASHERS UNDER BOLT HEAD AND NUT OR SPECIALLY SHAPED BOLTS AND NUTS. PURLIN BOLTS SHALL BE
- M12 4.6/S FOR SECTIONS UP TO 250 DEEP UNLESS NOTED OTHERWISE
 - M16 4.6/S FOR SECTIONS OVER 250 DEEP UNLESS NOTED OTHERWISE

- S15 CORROSION PROTECTION
THE FOLLOWING ARE THE MINIMUM REQUIREMENTS FOR PROTECTIVE TREATMENT. REFER TO THE ARCHITECTURAL SPECIFICATIONS FOR EXTRA FINISH COATS AND COLOURS. ALL COATINGS TO BE COMPATIBLE WITH APPLIED FINISHES INCLUDING TOP COAT AND ANY FIRE PROTECTION COATING.
PAINT REPAIRS SHALL BE CARRIED OUT TO GIVE THE SAME LEVEL OF PROTECTION AS THE ORIGINAL TREATMENT. ALL PAINT AND REPAIRS SHALL COMPLY WITH ANY SPECIFIED WARRANTY.

STEELWORK CONT'D

INTERNAL ENVIRONMENTS (EXCLUDING SPECIAL ENVIRONMENTS)			
Member	Surface Preparation To AS1627	Primer	Top coat
All U.N.O.	Power Tool Class 1 or Abrasive Blast class 1	Red Oxide Zinc Phosphate 75% m	To Architect's Specifications
External environments * Delete if not required *			
Member	Paint System to AS/NZS 2312 Table 6.3		
-	Corrosivity Category	Life span	
-	-	-	

HOT-DIP GALVANISING
UNLESS SPECIFIED OTHERWISE, UNDER ALL EXTERNAL ENVIRONMENTS ALL STRUCTURAL STEELWORK WHICH IS EXPOSED, OR IS IN CONTACT WITH EXPOSED BRICKWORK, AND ALL LINTELS SHALL BE HOT-DIP GALVANISED AFTER FABRICATION TO AS4680

Member	Hot-dip galvanised to AS4680
-	Normal finish
-	Architectural grade finish

ALL BOLTS, NUTS AND WASHERS, INCLUDING HOLDING-DOWN BOLTS SHALL BE GALVANISED TO AS1214.

- S16 CONCRETE-ENCASED STEELWORK IS TO BE WRAPPED WITH F41 MESH HAVING 50mm MINIMUM COVER OF CONCRETE GRADE N25 TO AS3600.

- S17 LOCATION OF PURLINS AND GIRTS TO BE OBTAINED FROM ARCHITECT'S DRAWINGS OR ROOFING CONTRACTOR.

- S18 PROVIDE SEAL PLATES TO ENDS OF ALL HOLLOW SECTIONS (WITH VENT HOT-DIP HOLES IF TO BE GALVANISED).

- S19 GRAVITY AND/OR GAUGE LINES TO INTERSECT, UNLESS NOTED OTHERWISE.

- S20 ROOF BRACING TO BE HOOK BOLTED TO EVERY SECOND PURLIN, OR SIMILAR, SO THAT BRACING IS STRAIGHT. BOLTS FOR HANGING DUCTS AND PIPES ETC FROM PURLINS SHALL BE ATTACHED TO THE WEB OF THE PURLIN, NOT THE FLANGE.

- S21 BASE PLATES SHALL BE GROUTED BEFORE MEMBER IS SUBSTANTIALLY LOADED. GROUT SHALL HAVE MINIMUM STRENGTH f'c OF 20 MPa AND SHALL BE DRYPACK MORTAR, RAMMED IN OR AN APPROVED NON-SHRINK GROUT.

- S22 ALL STEELWORK IS TO BE TEMPORARILY BUT SECURELY BRACED UNTIL ALL FINAL BRACING, CLADDING AND STABILISING BRICK OR BLOCKWORK HAS BEEN COMPLETED.

- S23 SHOP DRAWINGS SHALL BE PREPARED BY THE FABRICATOR FOR ALL STRUCTURAL STEELWORK. SUBMIT ALL WORKSHOP DRAWINGS TO STRUCTURAL ENGINEER FOR STRUCTURAL REVIEW AT LEAST FOURTEEN DAYS PRIOR TO FABRICATION. DO NOT FABRICATE STEELWORK UNTIL WORKSHOP DRAWINGS ARE APPROVED.

- S24 ALL FLASHING AND WATERPROOFING ELEMENTS SHALL BE AS SPECIFIED IN ARCHITECTURAL DOCUMENTS.

- S25 DRILLED-IN ANCHORS

- S25.1 DETAILS
DRILLED ANCHORS SHALL BE USED WHERE SHOWN ON THE DRAWINGS, OR WHERE PERMITTED IN WRITING BY THE ENGINEER. SUBMIT DETAILS OF PROPOSED ANCHORS, BEFORE USE, IN WRITING, TO THE ENGINEER FOR REVIEW. INSTALL ANCHORS IN ACCORDANCE WITH MANUFACTURER'S WRITTEN DIRECTIONS. TEST ANCHORS AS SPECIFIED IN 24.4

- S25.2 SPACING AND EDGE DISTANCES SHALL BE AS SHOWN, OR IN ACCORDANCE WITH THE MANUFACTURERS DIRECTIONS, AND SHALL BE APPROPRIATE FOR THE LOAD ON THE ANCHOR. UNLESS SHOWN OTHERWISE OR ALLOWED BY THE MANUFACTURER, THE FOLLOWING MINIMUMS SHALL BE USED FOR M20 CHEMICAL ANCHORES IN CONCRETE: SPACING=150mm, EDGE DISTANCE=150mm.

- S25.3 FOR ATTACHMENT TO HOLLOW MASONRY OR CONCRETE PANELS, USE HILTI HIT HY20 OR EQUIVALENT.

- S25.4 HOLES IN STEELWORK SHALL BE
- 2mm OVERSIZE WHEN THE STEEL IS TO BE USED AS A DRILLING TEMPLATE, OR
 - 6mm MAXIMUM OVERSIZE WHERE THE BOLTS ARE INSTALLED BEFOREHAND.

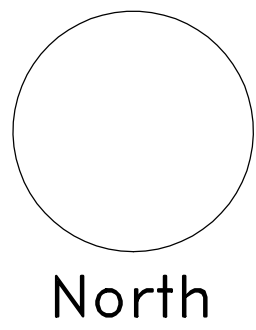
- S25.5 DRILLED-IN ANCHOR TESTING
ANCHOR TESTING
TESTING LOAD TO BE 150% OF SAFE WORKING LOAD OR 100% OF ULTIMATE LOAD, TO MANUFACTURER'S PRODUCT SPECIFICATION. TESTS TO BE CARRIED OUT BY N.A.T.A. REGISTERED LABORATORY AT THE CONTACTOR'S EXPENSE.
CHEMICAL ANCHORS
NUMBER OF CHEMICAL ANCHORS TO BE TESTED IS AS FOLLOWS: INSTALLATION FROM ABOVE AND SIDE = 20% OF TOTAL NUMBER IS TO BE TESTED.
INSTALLATION FROM BELOW = 100% OF TOTAL NUMBER IS TO BE TESTED
MECHANICAL ANCHORS
TEST 10% OF MECHANICAL ANCHORS
FAILURE
IF ONE ANCHOR IN A GROUP FAILS UNDER TESTING THEN ALL ANCHORS SHALL BE TESTED, AS SPECIFIED ABOVE, AT THE CONTRACTOR'S EXPENSE. ALL ANCHORS THAT FAIL ARE TO BE REPLACED AND RETESTED.

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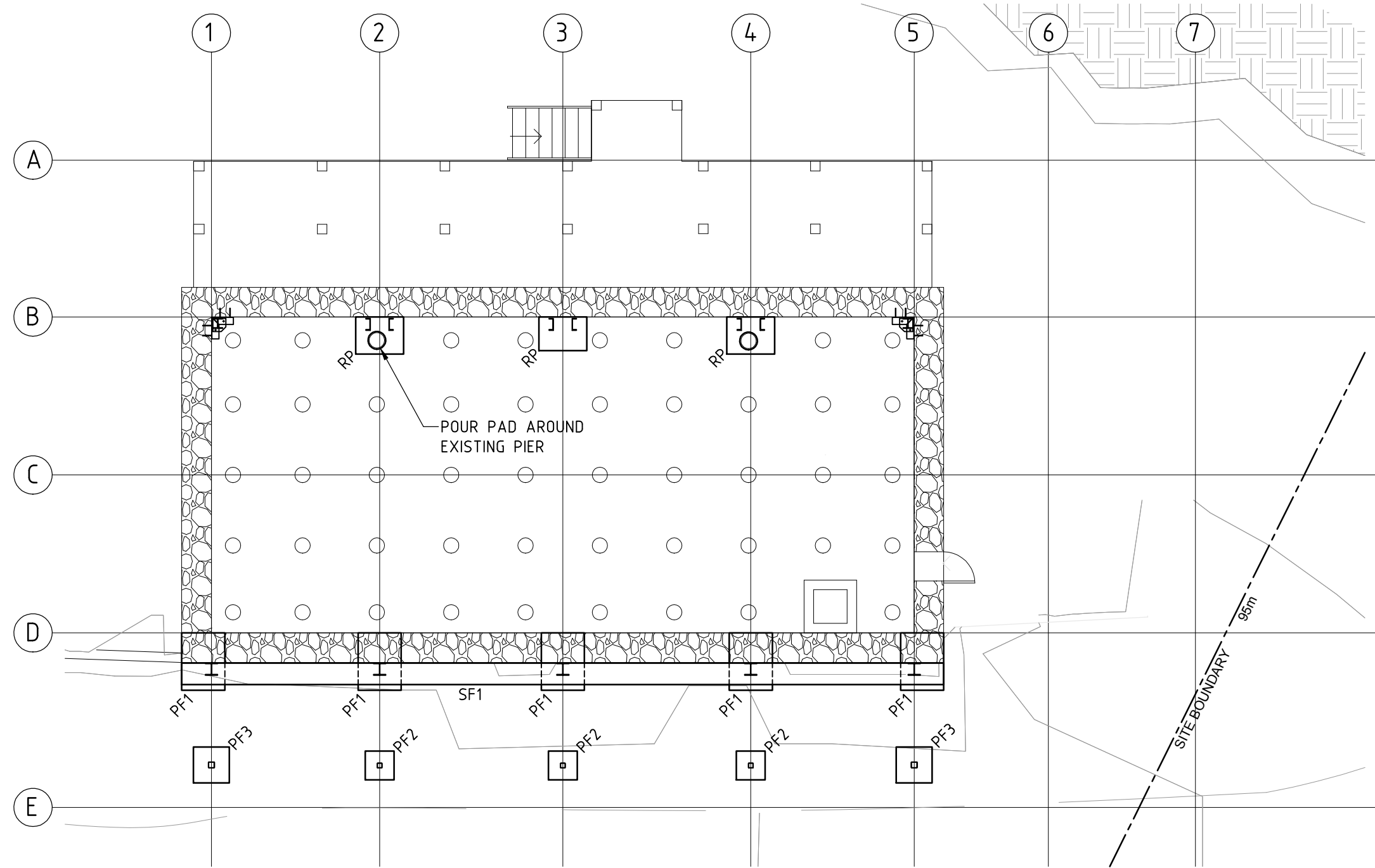
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Revision	Amendment or reason for issue	Issue date	Drawing Completed by	Designed & dwg. checked by	Verified by X = Not verified
					Issue authorised (*)



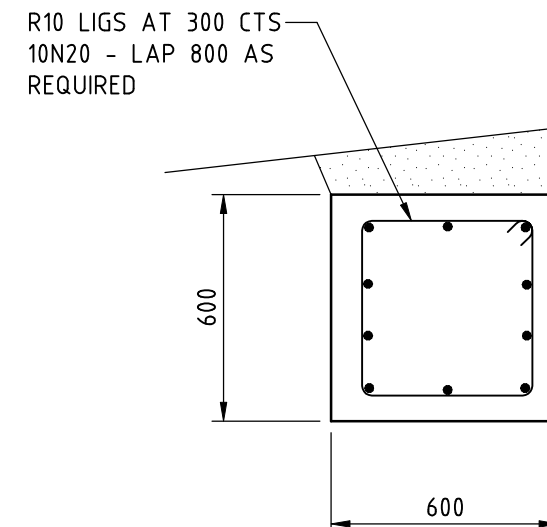
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	Client COOTAMUNDRA GUNDAGAI REGIONAL COUNCIL	Scales NTS	Client Project No.		
	Architect / Project Manager NOEL THOMSON ARCHITECTURE	Drawing No.	19S023-S02	Sheet 2 of 9	Revision -



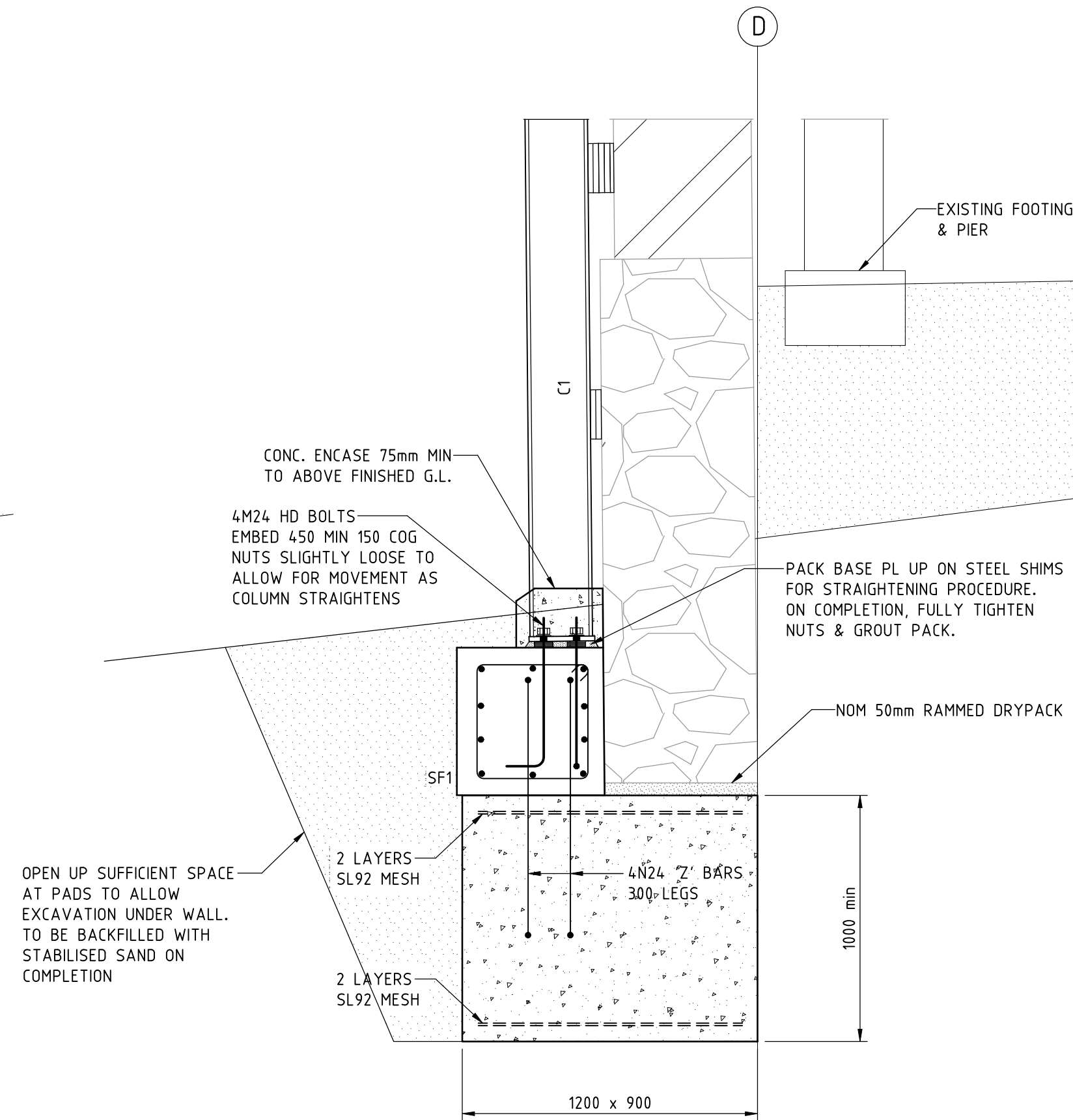
FOOTINGS PLAN - RECTIFICATION

FOOTING SCHEDULE

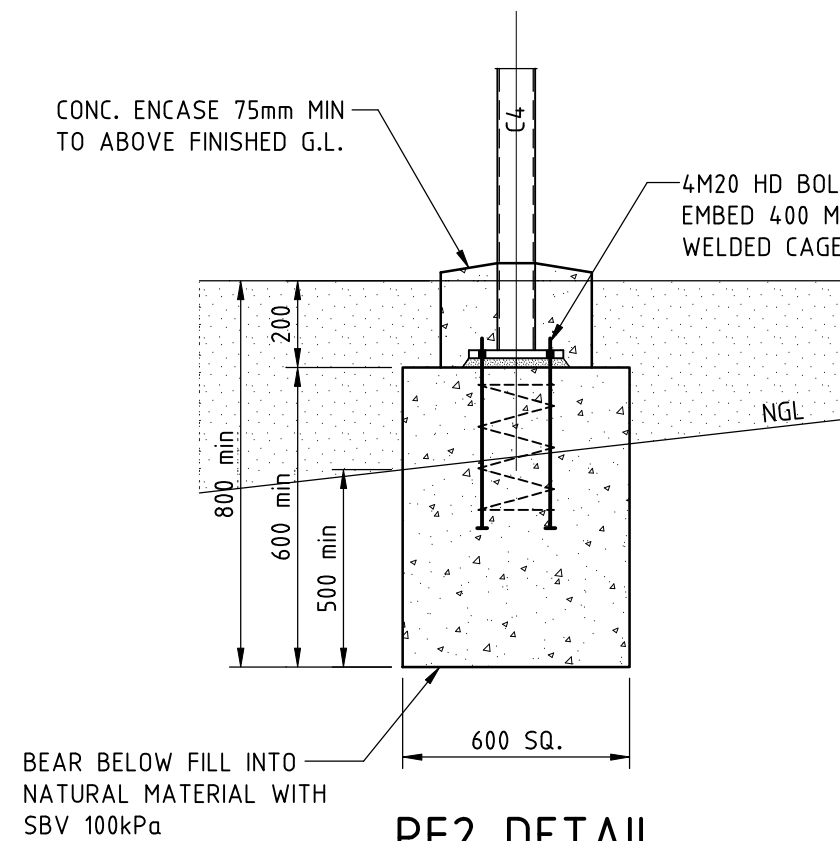
PAD FOOTING	PF1	1000 x 1200 x 1000 MIN DEEP
	PF2	600 SQUARE x 600 MIN DEEP
	PF3	750 SQUARE x 600 MIN DEEP
RECTIFICATION PAD	RP	1000 x 700 (MIN) x 500 MIN DEEP
STRIP FOOTING	SF1	600 W x 600 MIN DP POURED OVER PF1's



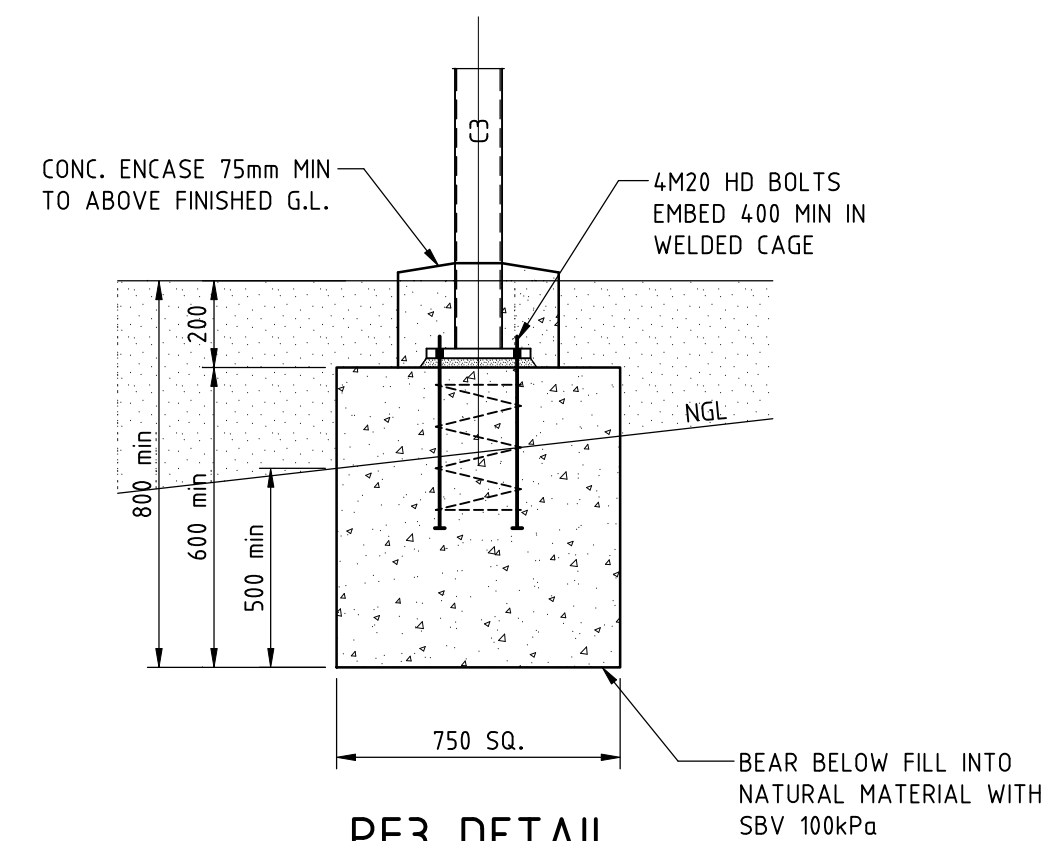
SF1 DETAIL



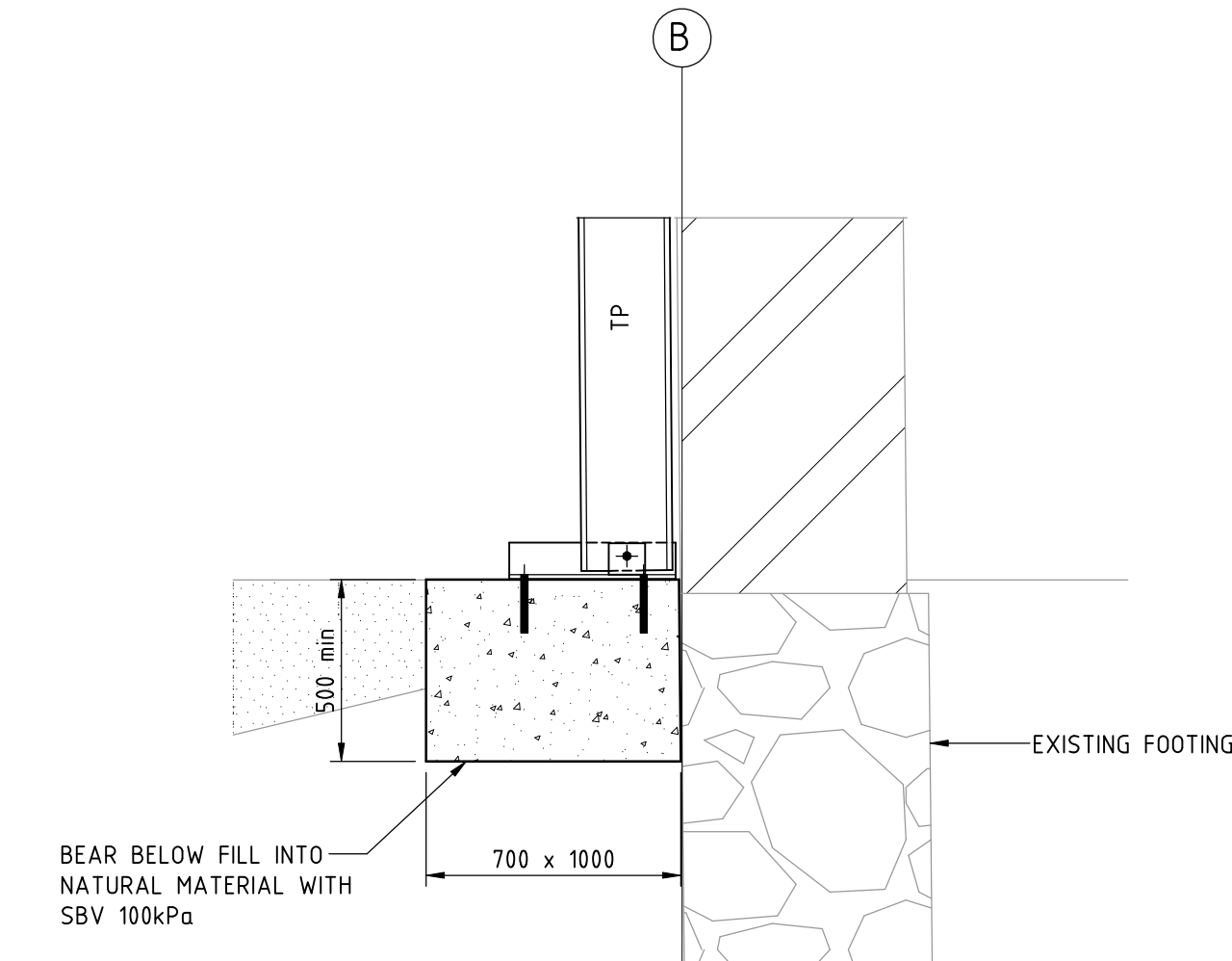
PF1 DETAIL



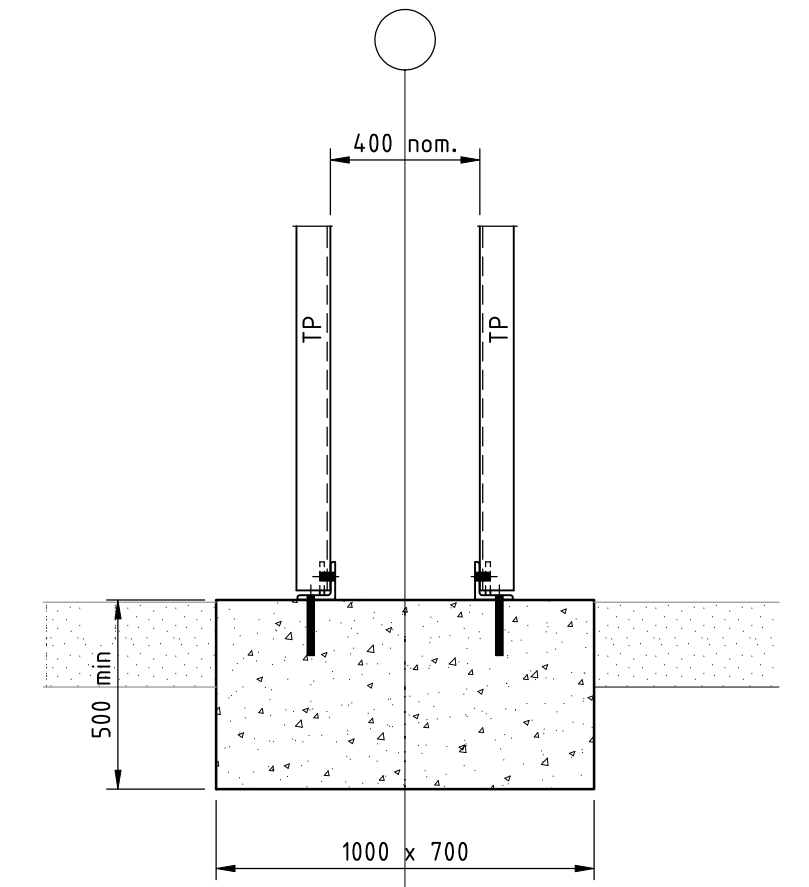
PF2 DETAIL
PAD TO BEAR 500 MIN INTO NATURAL GROUND



PF3 DETAIL
PAD TO BEAR 500 MIN INTO NATURAL GROUND



RP DETAIL



RP ELEVATION

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Project
OLD MILL GALLERY GUNDAGAI STAGE 1 - RECTIFICATION
SHERIDAN LANE
GUNDAGAI

Client
COOTAMUNDRA GUNDAGAI REGIONAL COUNCIL

Architect / Project Manager
NOEL THOMSON ARCHITECTURE

Drawing Title
FOOTINGS PLAN & DETAILS

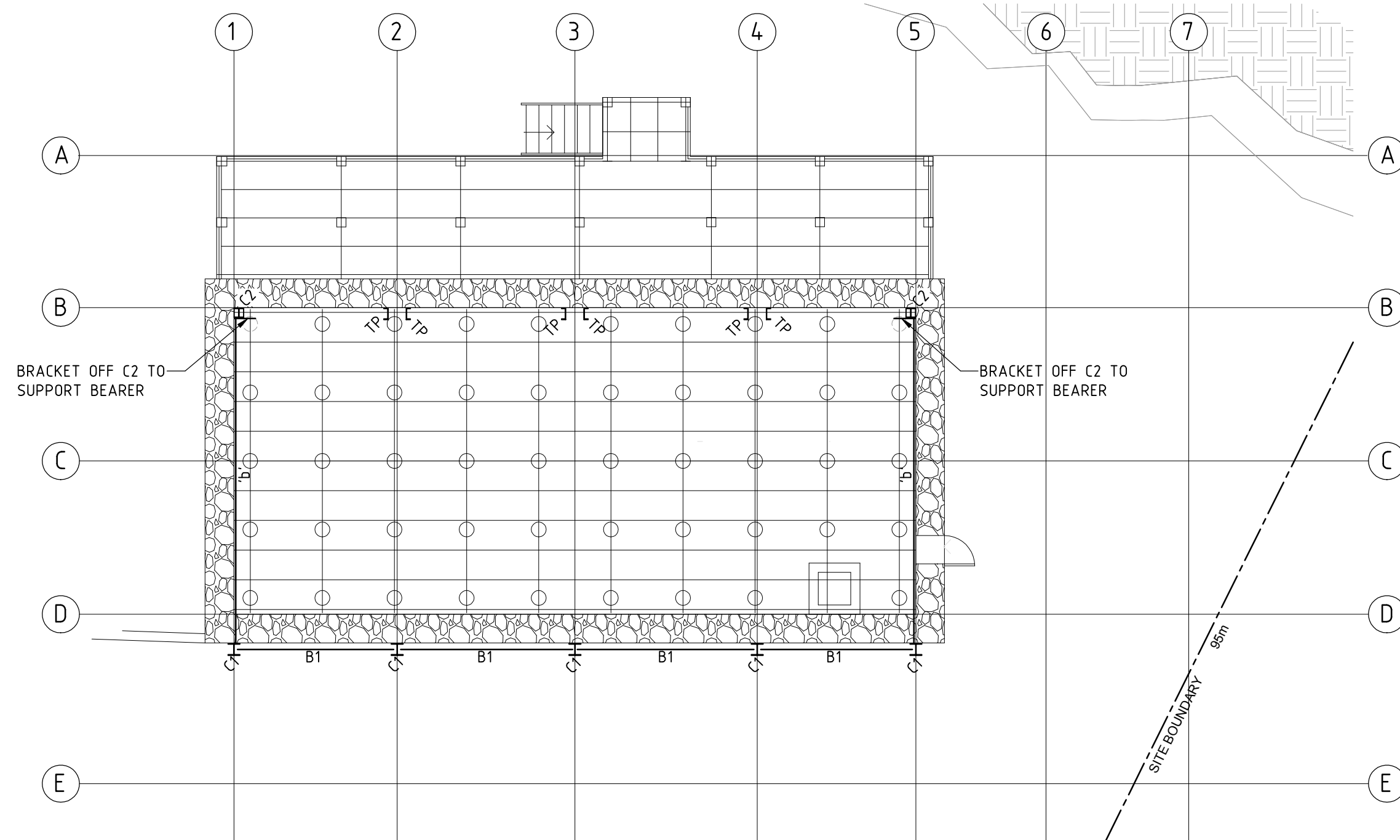
Scales
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Drawing No.
19S023-S03

Client Project No.

Sheet
3 of 9

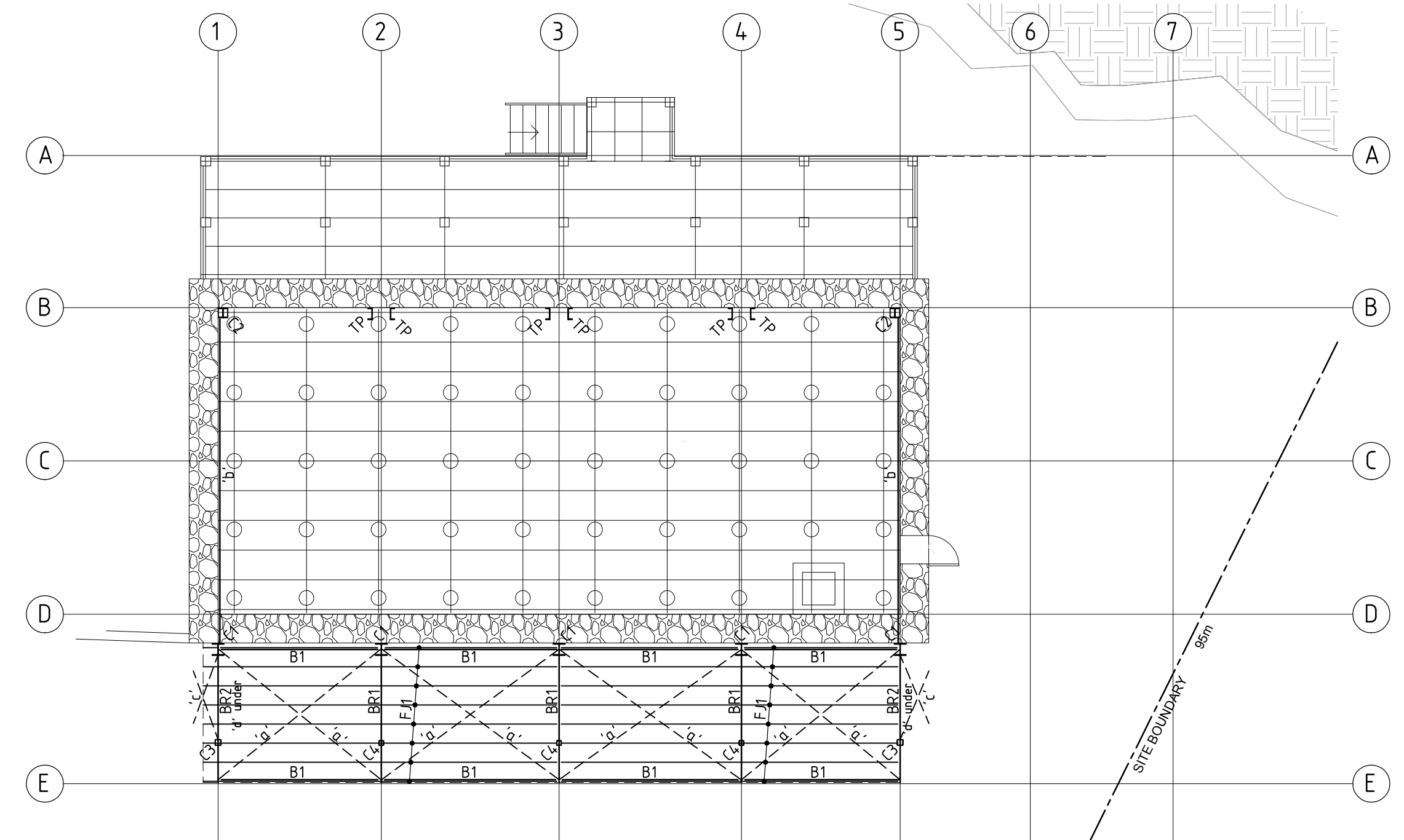
Revision
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GROUND FLOOR PLAN - RECTIFICATION

MEMBER SCHEDULE

COLUMN	C1	250UC73
	C2	200UC60
TEMPORARY POST	TP	250PFC
BEAM	B1	150PFC
BRACING	'b'	75x8 EA



GROUND FLOOR PLAN - STABILISATION

MEMBER SCHEDULE

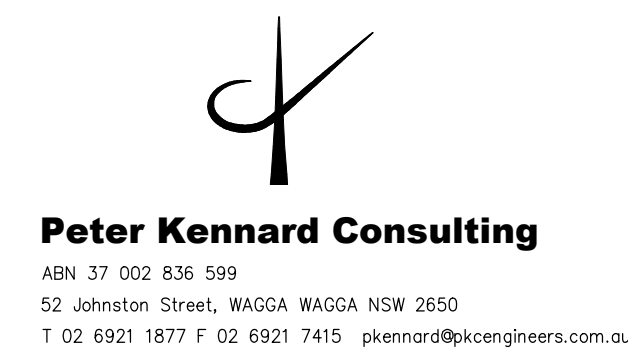
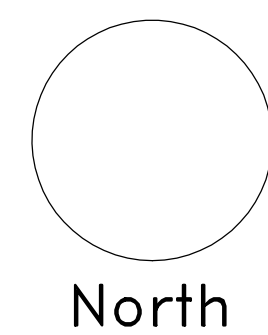
COLUMN	C1	250UC73 (GALV)
	C2	200UC60
	C3	125x5 SHS
	C4	100x4 SHS
TEMPORARY POST	TP	250PFC
BEARER	BR1	150x100x4 RHS
	BR2	150PFC
BEAM	B1	150PFC
FLOOR JOIST	FJ1	150x50x3 RHS AT 450 CTS MAX CONTINUOUS OVER 2 SPANS
BRACING	'a'	75x8 EA
	'b'	75x8 EA
	'c'	75x8 EA
	'd'	100x4 SHS

NOTE:
DECK MUST BE CONSTRUCTED & COMPLETE PRIOR
TO REMOVAL OF ANY TEMPORARY MEMBERS USED
IN STRAIGHTENING PROCEDURE.

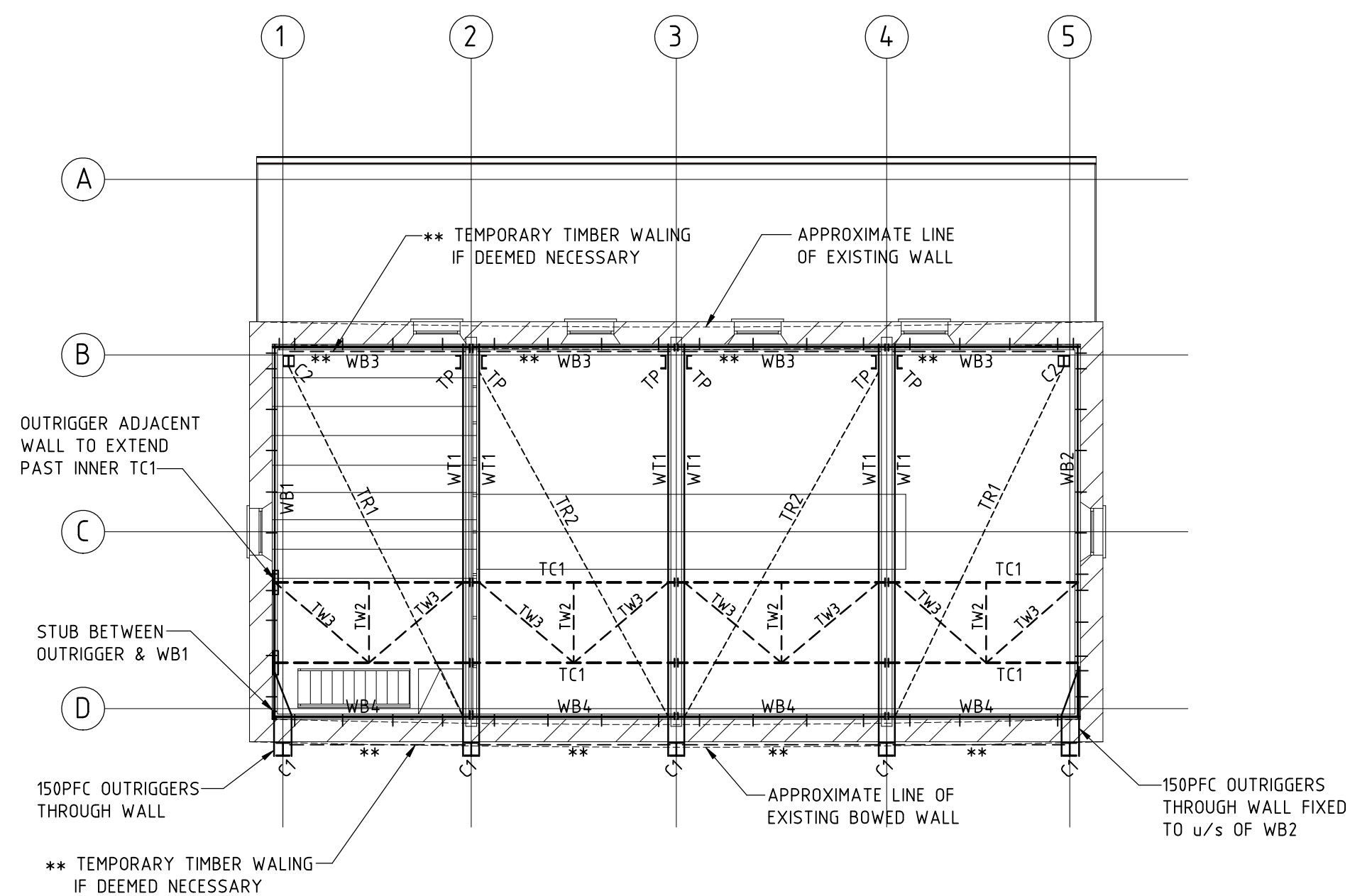
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Structural Engineers	Project OLD MILL GALLERY GUNDAGAI STAGE 1 - RECTIFICATION SHERIDAN LANE GUNDAGAI	Drawing Title GROUND FLOOR PLANS
Client COOTAMUNDRA GUNDAGAI REGIONAL COUNCIL	Architect / Project Manager NOEL THOMSON ARCHITECTURE	Scales 1:100
	Drawing No. 19S023-S04	Client Project No.
		Sheet 4 of 9
		Revision -



LEVEL 2 - RECTIFICATION & STRAIGHTENING PLAN

MEMBER SCHEDULE

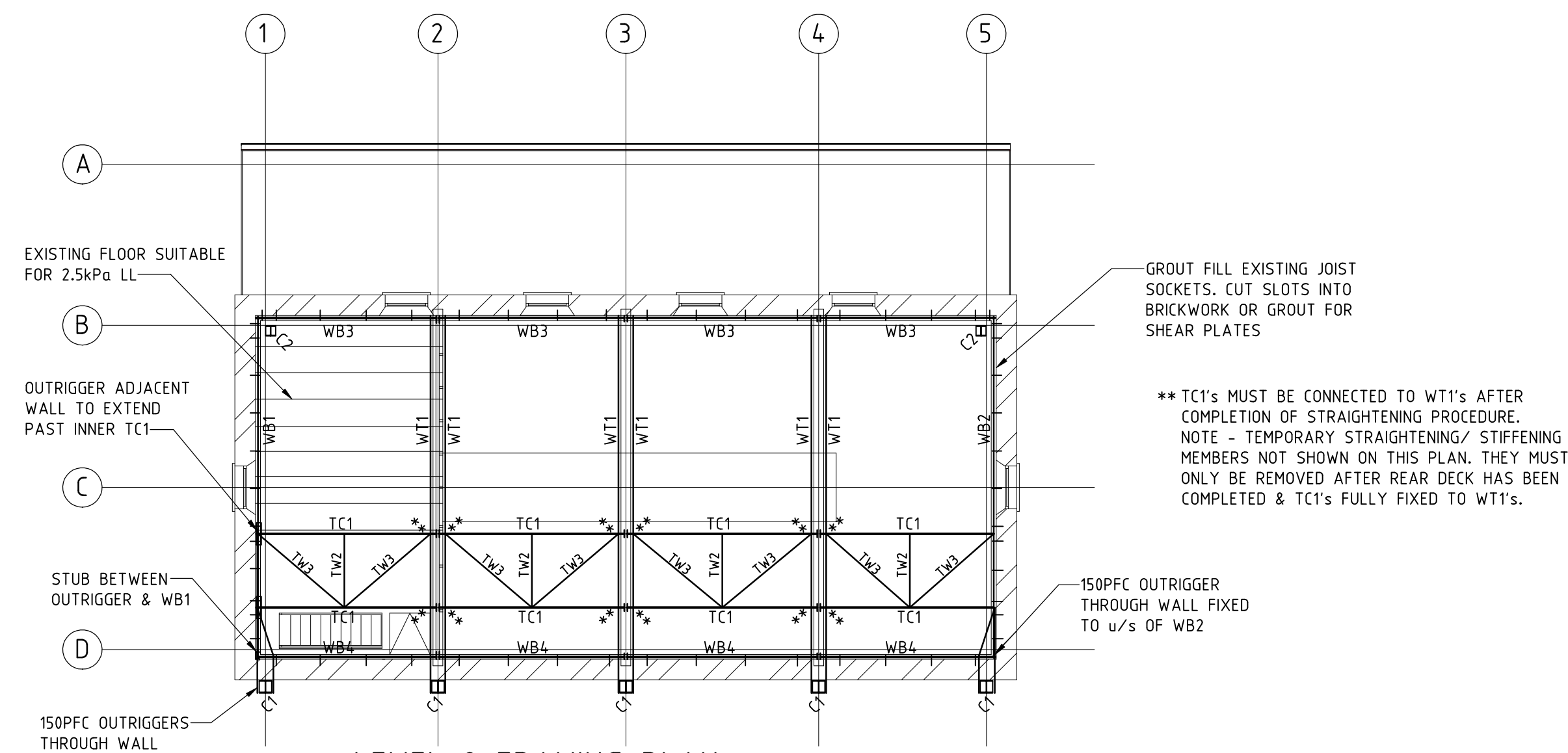
COLUMN	C1	250UC73
	C2	200UC60
	C3	125x5 SHS
	C4	100x4 SHS
TEMPORARY POST	TP	250PFC
WALL TIE	WT1	150PFC's BOLTED TO EXISTING BEAMS WITH M20 THROUGH BOLTS AT 1200 CTS MAX PLUS EXTRA BOLTS AS NOTED
WALL BEAM	WB1	200PFC FIXED TO WALL ABOVE FLOOR LEVEL WITH DP20160 DYNABOLTS AT 1000 CTS MAX & ADDITIONAL FIXINGS AS NOTED
	WB2	200PFC FIXED TO WALL AT JOIST LEVEL WITH DP20160 DYNABOLTS AT 1000 CTS MAX & ADDITIONAL FIXINGS AS NOTED
	WB3	150x100x10 UA - FIXED TO WALL WITH DP20160 DYNABOLTS IN 40mm HORIZONTALLY SLOTTED HOLES AT 1000 CTS MAX & ADDITIONAL FIXINGS AS NOTED
	WB4	150x100x10 UA - FIXED TO WALL WITH DP20160 DYNABOLTS IN 40mm HORIZONTALLY SLOTTED HOLES AT 1000 CTS MAX & ADDITIONAL FIXINGS AS NOTED
TRUSS CHORD	TC1	200x100x5 RHS AT JOIST LEVEL (ABOVE EXISTING BEAMS)
TRUSS WEB	TW1	150PFC AT SAME LEVEL AS WT1's
	TW2	150PFC AT SAME LEVEL AS WT1's
	TW3	150PFC AT SAME LEVEL AS WT1's
TEMPORARY POST	TP	USED DURING STRAIGHTENING PROCEDURE, REMOVED ON COMPLETION
TENSION ROD	TR1	24Ø ROD WITH TURNBUCKLE
	TR2	24Ø ROD WITH TURNBUCKLE

PROPOSED STRAIGHTENING PROCEDURE

SUGGESTED PROCEDURE ONLY, TO BE REFINED OR ALTERED IN CONSULTATION WITH CONTRACTOR.

- OPEN UP REQUIRED FLOOR AREAS OF EXISTING LEVELS 1 & 2, ENSURING ALL UNAFFECTED AREAS REMAIN SUPPORTED THROUGHOUT PROCEDURE. SOME TOLERANCE BETWEEN THE LEVEL 1 FLOOR FRAMING & SOUTHERN SIDE BASE WALL IS REQUIRED.
- EXCAVATE & INSTALL PAD FOOTINGS FOR C1's C2's & PADS FOR TEMPORARY POSTS.
INSTALL ALL STEELWORK DIRECTLY REQUIRED FOR THE STRAIGHTENING & GENERAL RECTIFICATION OF THE EXISTING BUILDING (NOT EXTERNAL DECK FRAMING).
NOTE THAT THE STIFFENING TRUSS AT LEVEL 2 IS TO BE INSTALLED, BUT FIXED AT END WALLS ONLY. INTERNAL TRUSS FIXING POINTS TO WT1's TO BE LEFT FREE TO ALLOW MOVEMENT DURING STRAIGHTENING PROCEDURE. NOTCHES REQUIRED IN TIMBER BEAMS TO INSTALL STIFFENING TRUSS, MUST ALLOW FOR THE REQUIRED MOVEMENT.
- WHEN ALL STEELWORK & ANY ADDITIONAL STIFFENING OR SUPPORT MEMBERS & PACKING HAVE BEEN INSTALLED, INSPECTED & APPROVED, TENSION RODS TR1 & TR2 TO BE INITIALLY LOADED (SNUG TIGHT). IT IS RECOMMENDED THAT OBSERVERS ARE POSITIONED AROUND THE BUILDING DURING STRAIGHTENING & THE STRUCTURAL ENGINEER BE PRESENT.
- INITIAL TENSIONING TO BE DONE WITH THE 2 INNER RODS (TR2's), THEN THE 2 OUTER RODS (TR1's). ALTERNATING BETWEEN INNER & OUTER RODS AS EACH INCREMENTAL MOVEMENT IS ACHIEVED. SIZE OF EACH INCREMENTAL MOVE TO BE DETERMINED ON SITE.
- WHEN REQUIRED STRAIGHTENING HAS BEEN ACHIEVED, OR ABORTED DUE TO VISIBLE DAMAGE OCCURRING OR THE BUILDING'S REFUSAL TO MOVE, FINAL FIXING OF LEVEL 2 TRUSS CAN TAKE PLACE.
THE SOUTH SIDE, GROUND FLOOR DECK IS CRITICAL TO STABILISING THE STRUCTURE. IT IS TO BE CONSTRUCTED WHILE TEMPORARY STIFFENING MEMBERS & TENSION RODS REMAIN IN PLACE.
- WHEN LEVEL 2 TRUSS HAS BEEN FIXED IN POSITION & SOUTH SIDE DECK HAS BEEN CONSTRUCTED, TENSION RODS CAN BE RELEASED SLOWLY & EVENLY WHILE KEEPING THE WALLS UNDER OBSERVATION. ENGINEER SHOULD BE PRESENT DURING THIS PROCEDURE.
- ONCE TENSION HAS BEEN RELEASED, TENSION RODS, TEMPORARY POSTS & STIFFENING MEMBERS CAN BE REMOVED, & ALL GENERAL REPAIRS, INTERNAL WORKS & ADDITIONAL EXTERNAL DECKS & RAMPS COMMENCED.

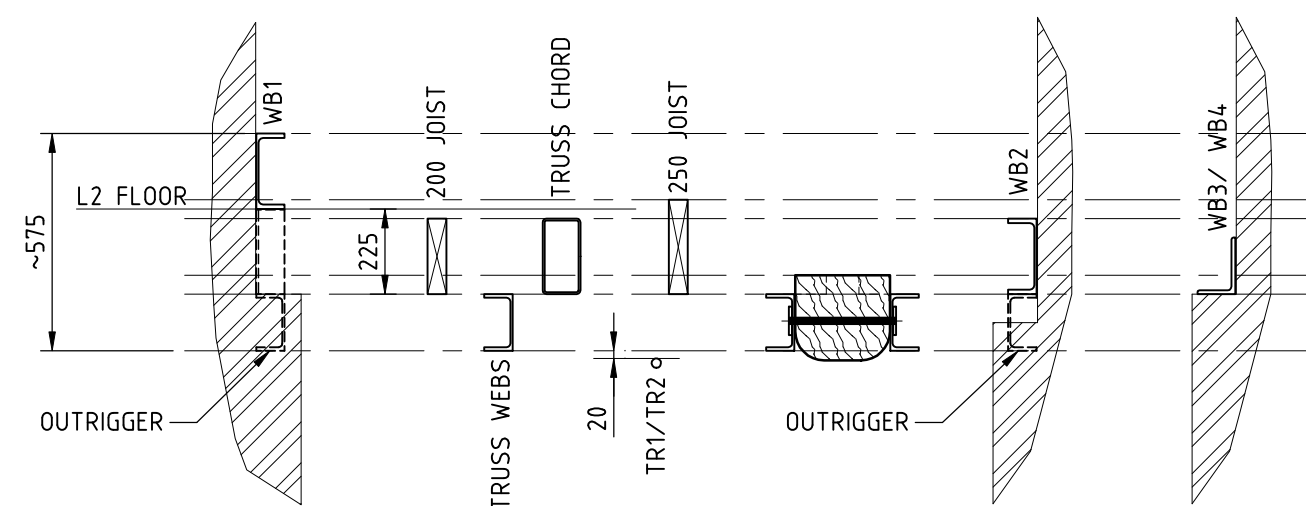
NOTE:
TEMPORARY STRAIGHTENING/ STIFFENING MEMBERS SHOWN ON THIS PLAN, AS WELL AS ANY ADDITIONAL MEMBERS DEEMED NECESSARY ON SITE, MUST REMAIN IN PLACE UNTIL AFTER TC1's HAVE BEEN FULLY FIXED TO WT1's & REAR DECK HAS BEEN COMPLETED.



LEVEL 2 FRAMING PLAN

MEMBER SCHEDULE

COLUMN	C1	250UC73
	C2	200UC60
	C3	125x5 SHS
	C4	100x4 SHS
WALL TIE	WT1	150PFC's BOLTED TO EXISTING BEAMS WITH M20 THROUGH BOLTS AT 1200 CTS MAX PLUS EXTRA BOLTS AS NOTED
WALL BEAM	WB1	200PFC FIXED TO WALL ABOVE FLOOR LEVEL WITH DP20160 DYNABOLTS AT 900 CTS MAX & ADDITIONAL FIXINGS AS NOTED
	WB2	200PFC FIXED TO WALL AT JOIST LEVEL WITH DP20160 DYNABOLTS AT 900 CTS MAX & ADDITIONAL FIXINGS AS NOTED
	WB3	150x100x10 UA - FIXED TO WALL WITH DP20160 DYNABOLTS IN 40mm HORIZONTALLY SLOTTED HOLES AT 900 CTS MAX & ADDITIONAL FIXINGS AS NOTED
	WB4	150x100x10 UA - FIXED TO WALL WITH DP20160 DYNABOLTS IN 40mm HORIZONTALLY SLOTTED HOLES AT 900 CTS MAX & ADDITIONAL FIXINGS AS NOTED
TRUSS CHORD	TC1	200x100x5 RHS AT JOIST LEVEL (ABOVE EXISTING BEAMS)
TRUSS WEB	TW1	150PFC AT SAME LEVEL AS WT1's
	TW2	150PFC AT SAME LEVEL AS WT1's
	TW3	150PFC AT SAME LEVEL AS WT1's

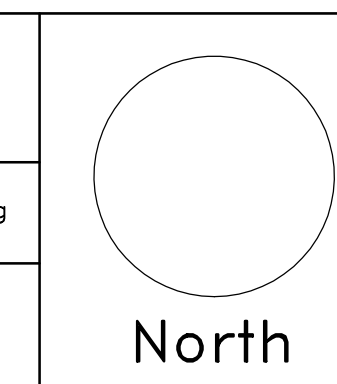


MEMBER LEVEL KEY

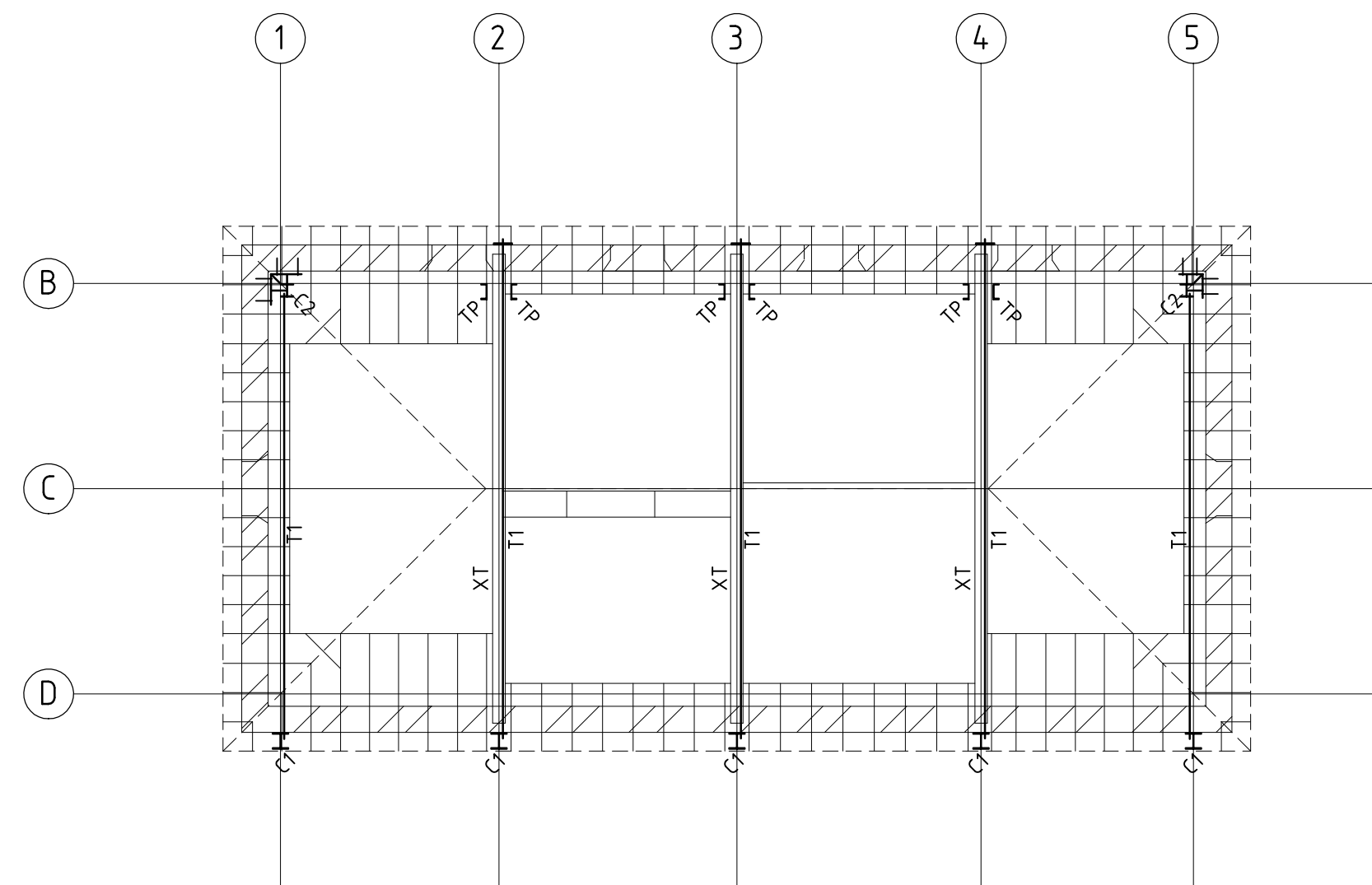
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Revision	Amendment or reason for issue	Issue date	C.A.	P.J.K.	X	Issue authorised (*)
-	ISSUED FOR INFORMATION	30.3.20				

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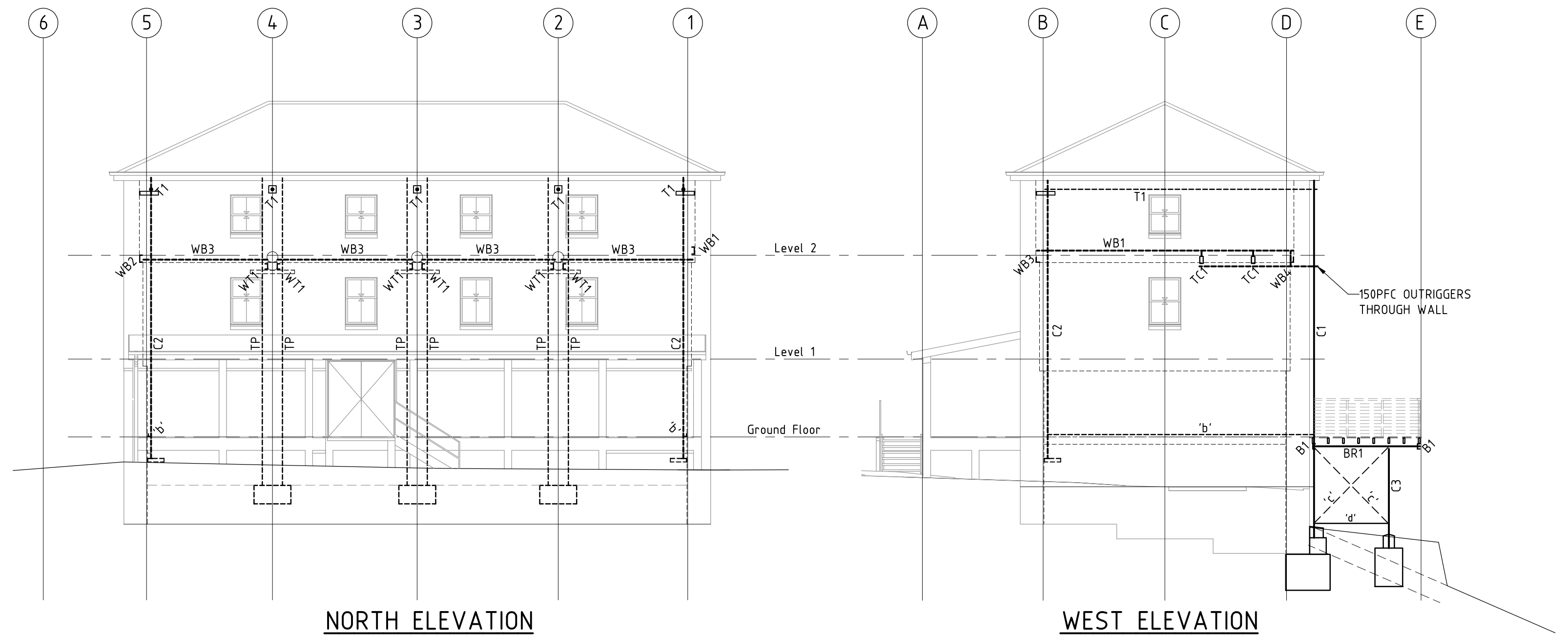
Project OLD MILL GALLERY GUNDAGAI STAGE 1 - RECTIFICATION SHERIDAN LANE GUNDAGAI	Drawing Title LEVEL 2 PLANS
Client COOTAMUNDRA GUNDAGAI REGIONAL COUNCIL	Scales 1:100
Architect / Project Manager NOEL THOMSON ARCHITECTURE	Client Project No.
Drawing No. 19S023-S05	Sheet 5 of 9
	Revision -



PLAN AT ROOF LEVEL

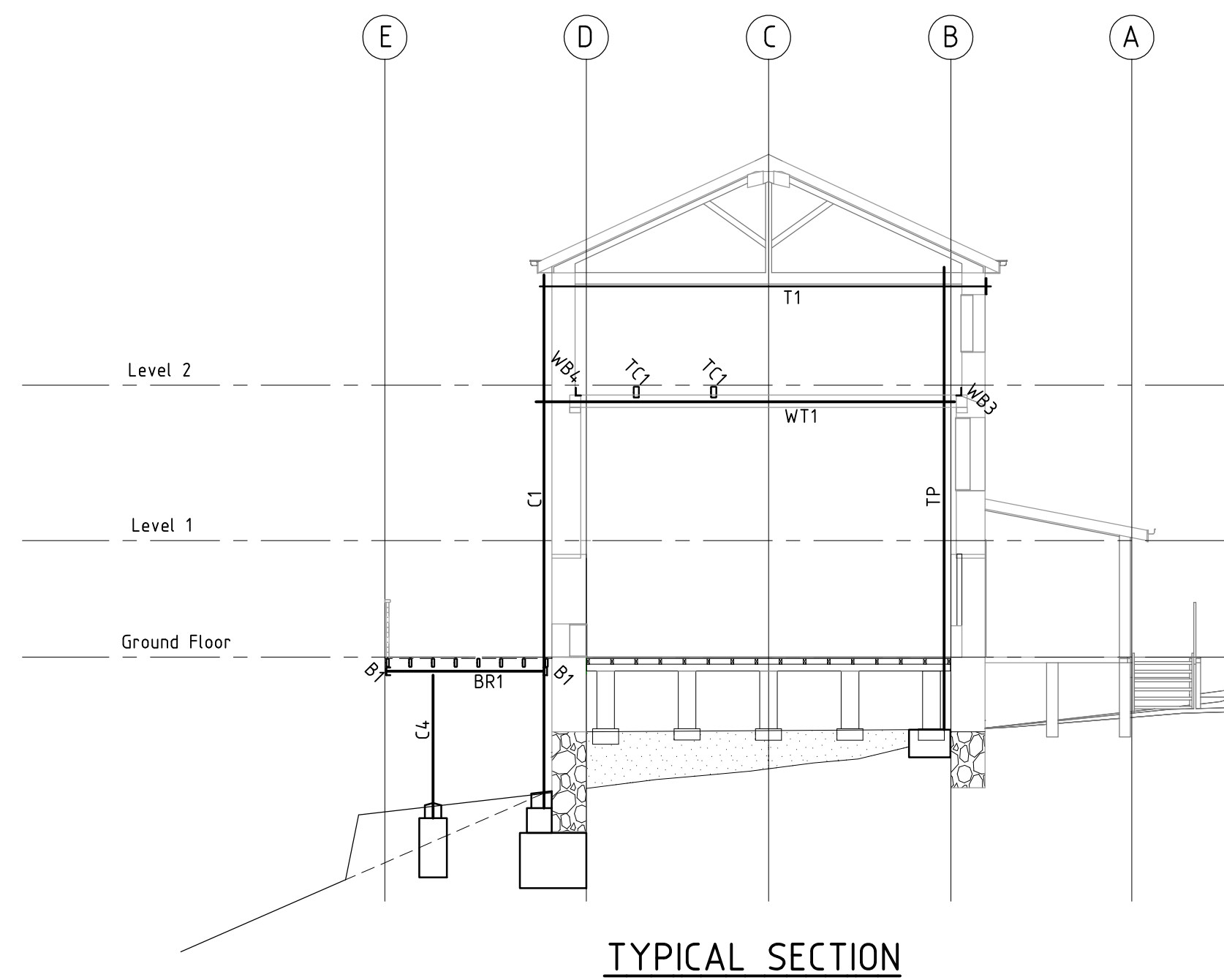
MEMBER SCHEDULE

COLUMN	C1	250UC73
	C2	200UC59
TIE ROD	T1	250 ROD WITH THREADED ENDS & TURNBUCKLES
EXISTING TRUSS	XT	EXISTING TIMBER ROOF TRUSS

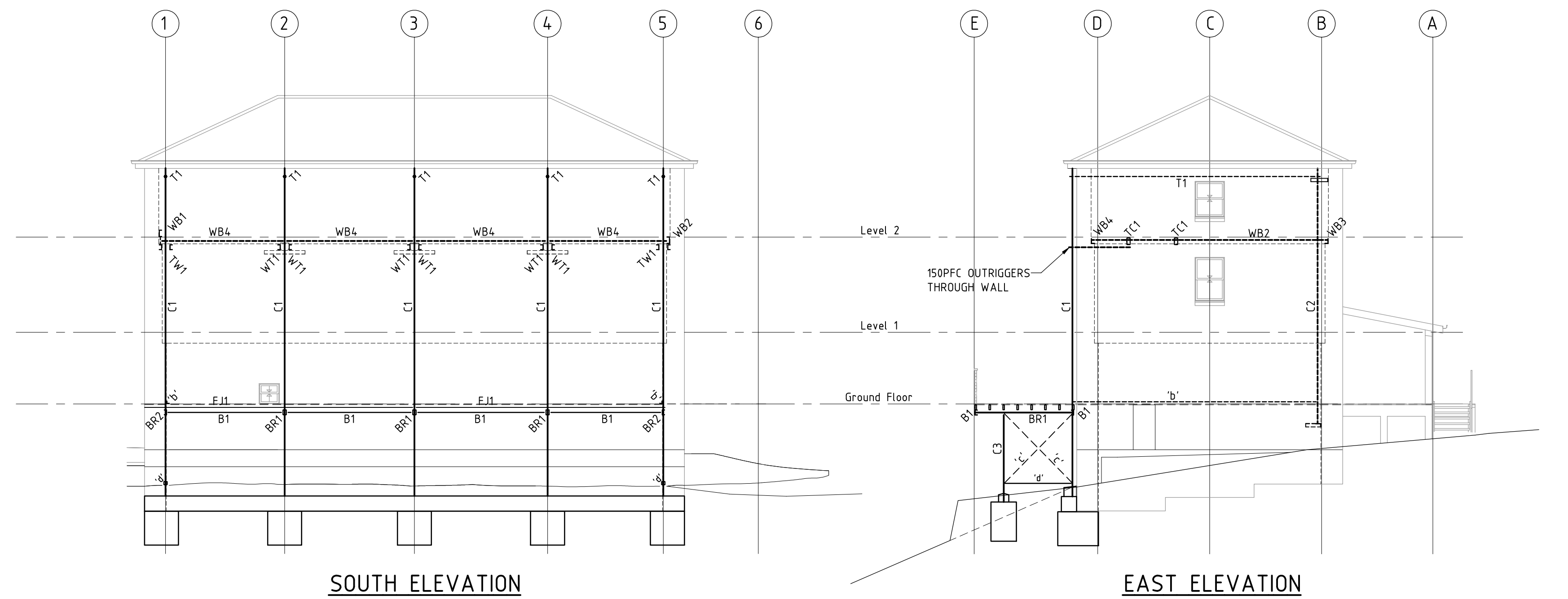


NORTH ELEVATION

WEST ELEVATION



TYPICAL SECTION



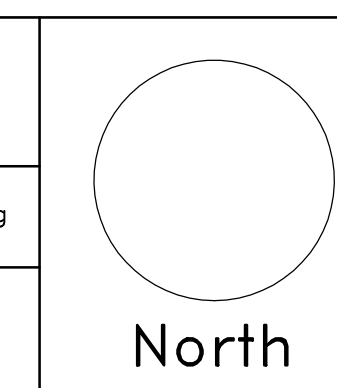
SOUTH ELEVATION

EAST ELEVATION

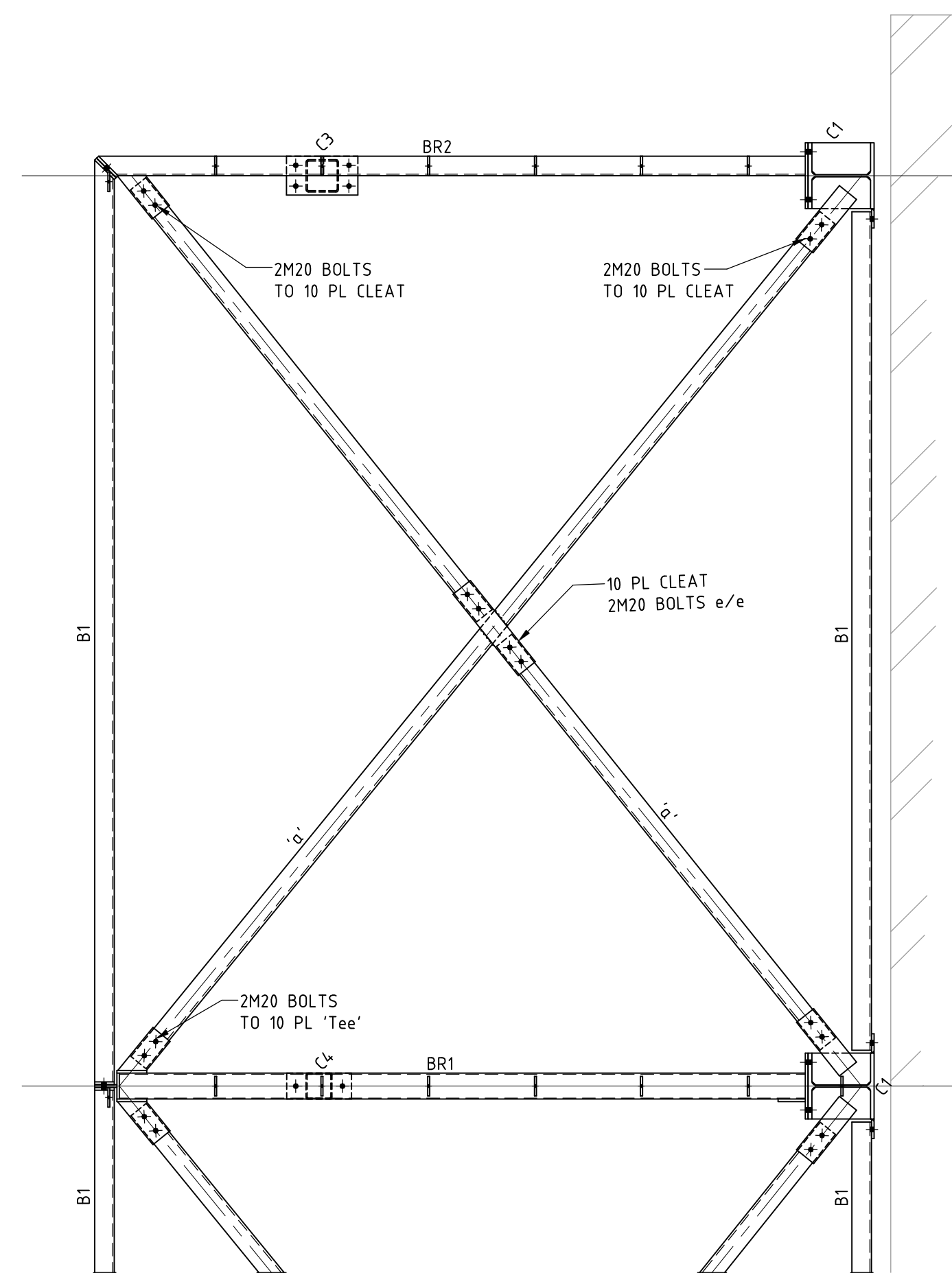
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Revision	Amendment or reason for issue	Issue date	C.A.	P.J.K.	X	Issue authorised (*)
-	ISSUED FOR INFORMATION	30.3.20	C.A.	P.J.K.	X	

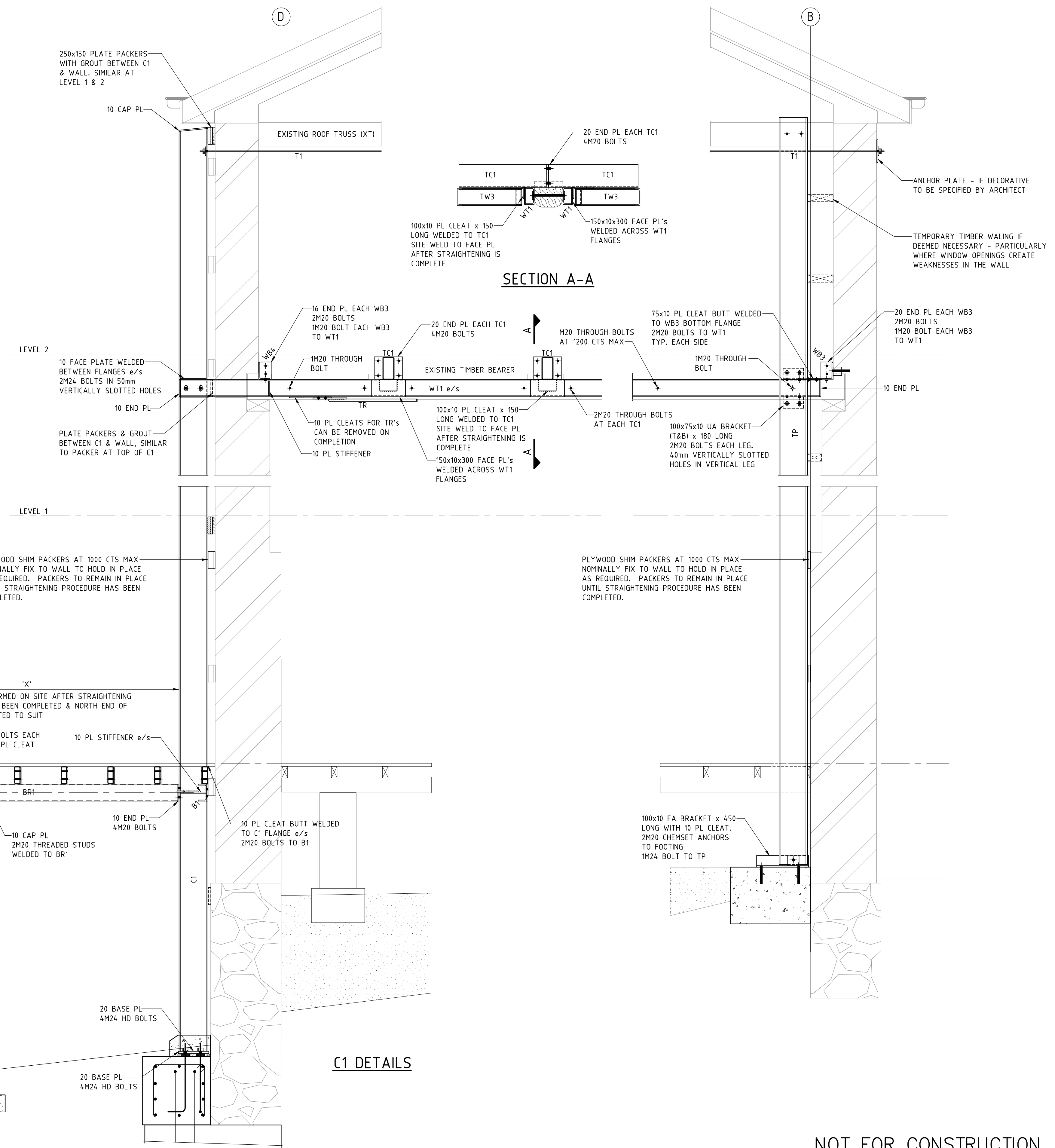
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Client COOTAMUNDRA GUNDAGAI REGIONAL COUNCIL	Architect / Project Manager NOEL THOMSON ARCHITECTURE	Scales 1:100
		Client Project No.
		Drawing No. 19S023-S06
		Sheet 6 of 9
		Revision -



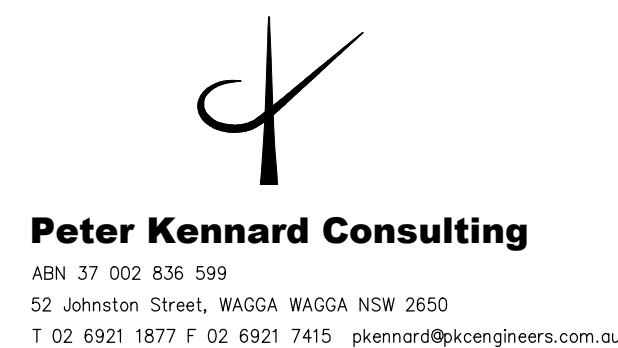
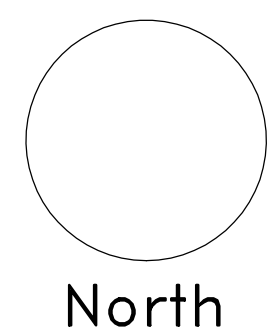
DECK BRACING DETAIL



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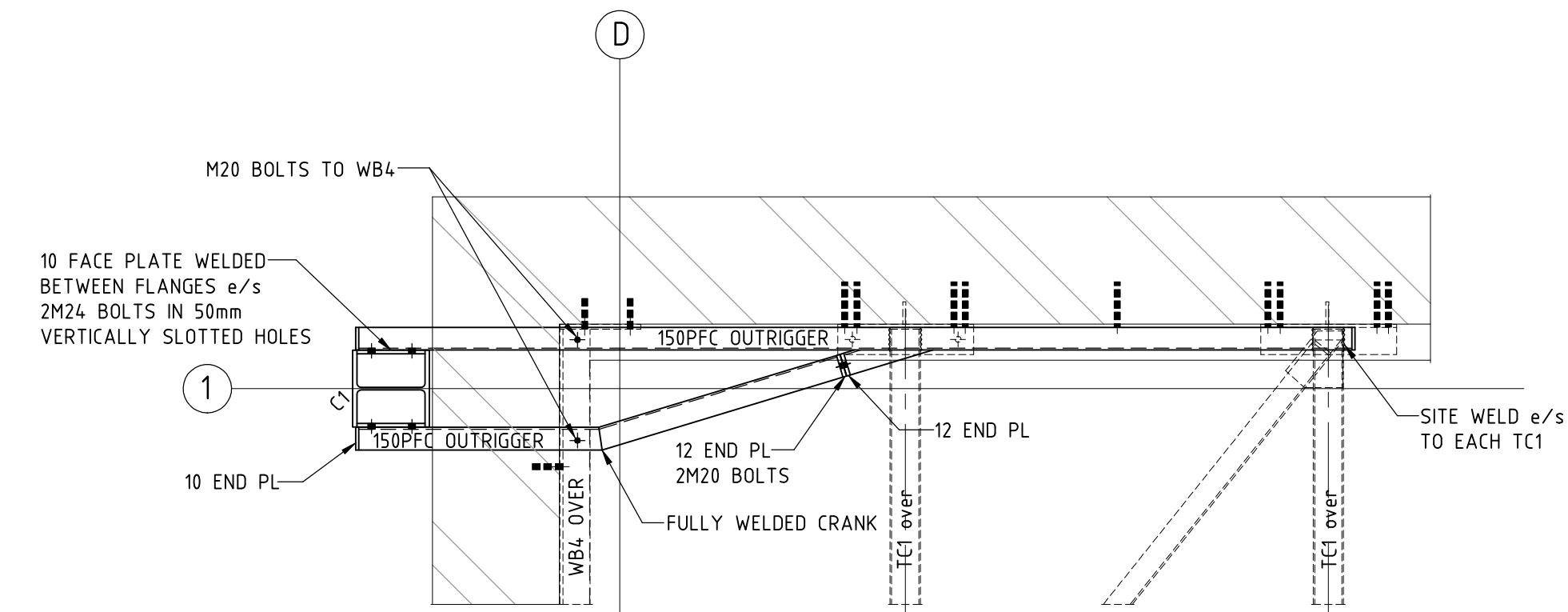
-	ISSUED FOR INFORMATION	30.3.20	C.A.	P.J.K.	X		
Revision	Amendment or reason for issue	Issue date	Drawing Completed by	Designed & dwg. checked by	Verified by X = Not verified	Issue authorised (*)	

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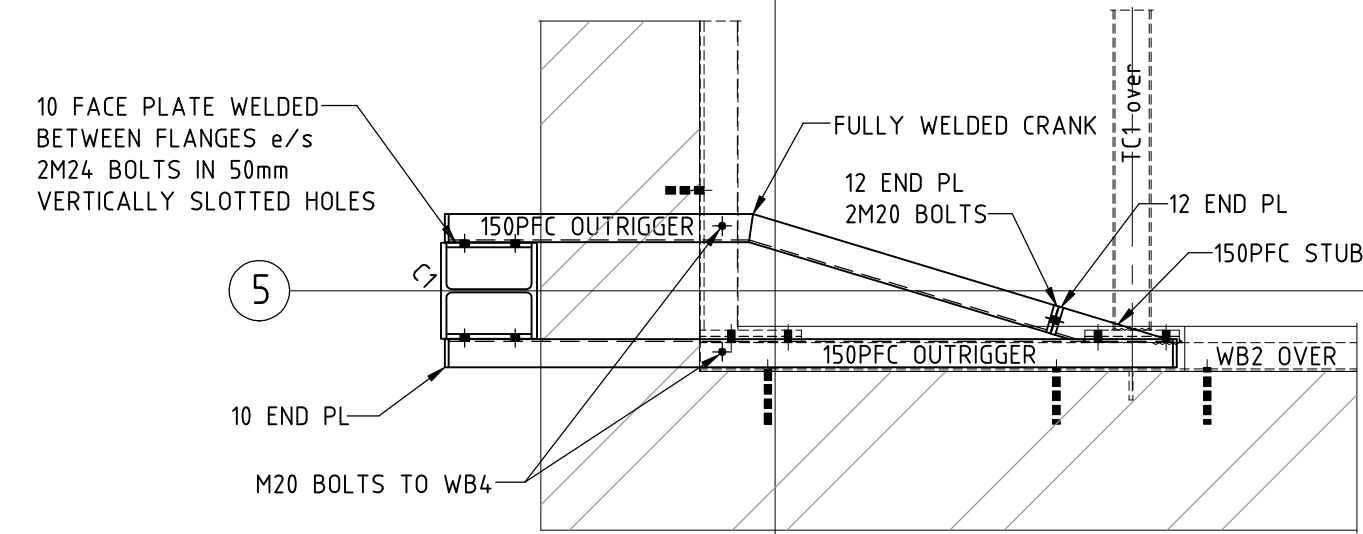


Structural Engineers	Project OLD MILL GALLERY GUNDAGAI STAGE 1 - RECTIFICATION SHERIDAN LANE GUNDAGAI	Drawing Title STEELWORK DETAILS SHEET 1		
	Client COOTAMUNDRA GUNDAGAI REGIONAL COUNCIL	Scales 1:20	Client Project No.	
	Architect / Project Manager NOEL THOMSON ARCHITECTURE	Drawing No. 19S023-S07	Sheet 7 of 9	Revision -

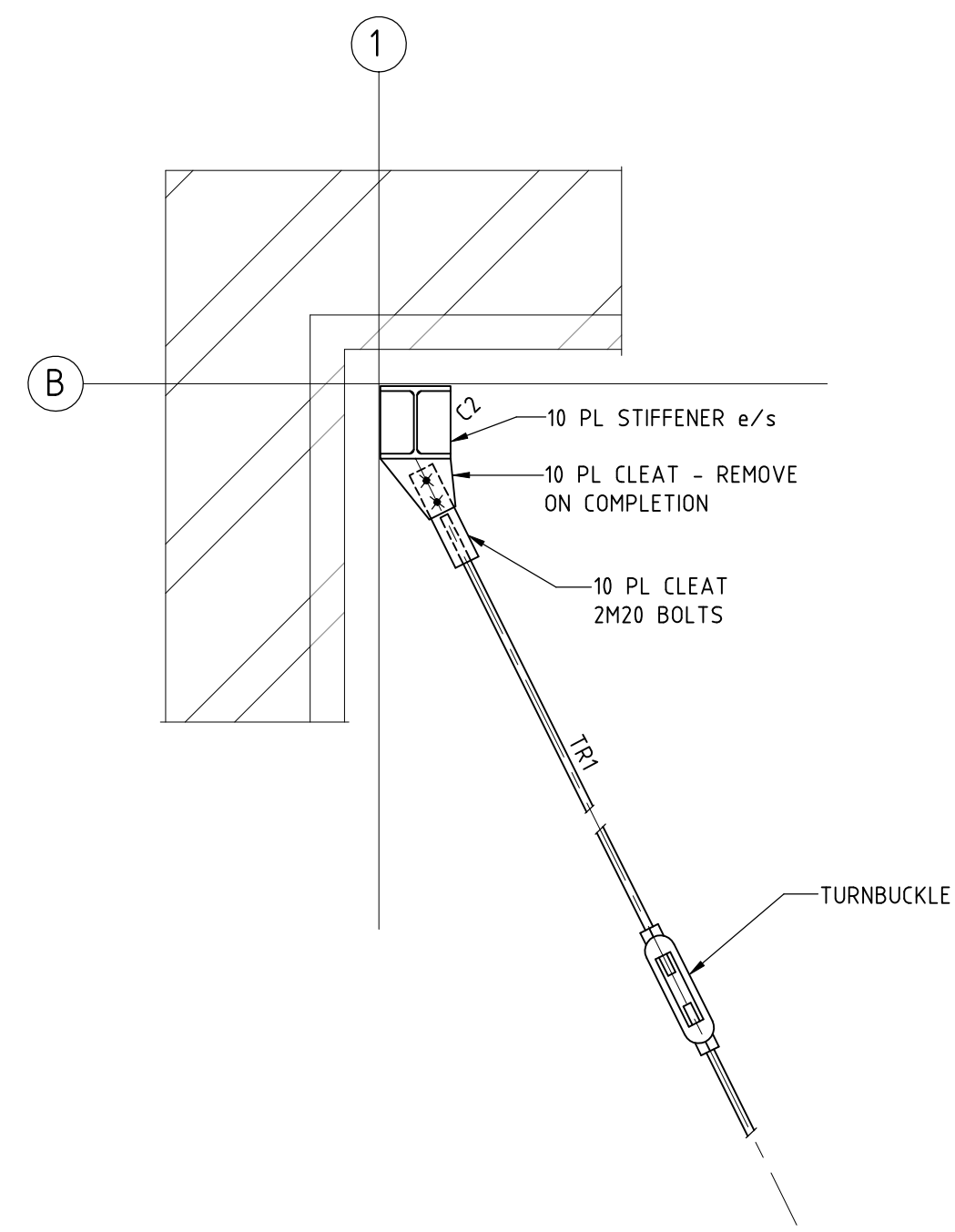
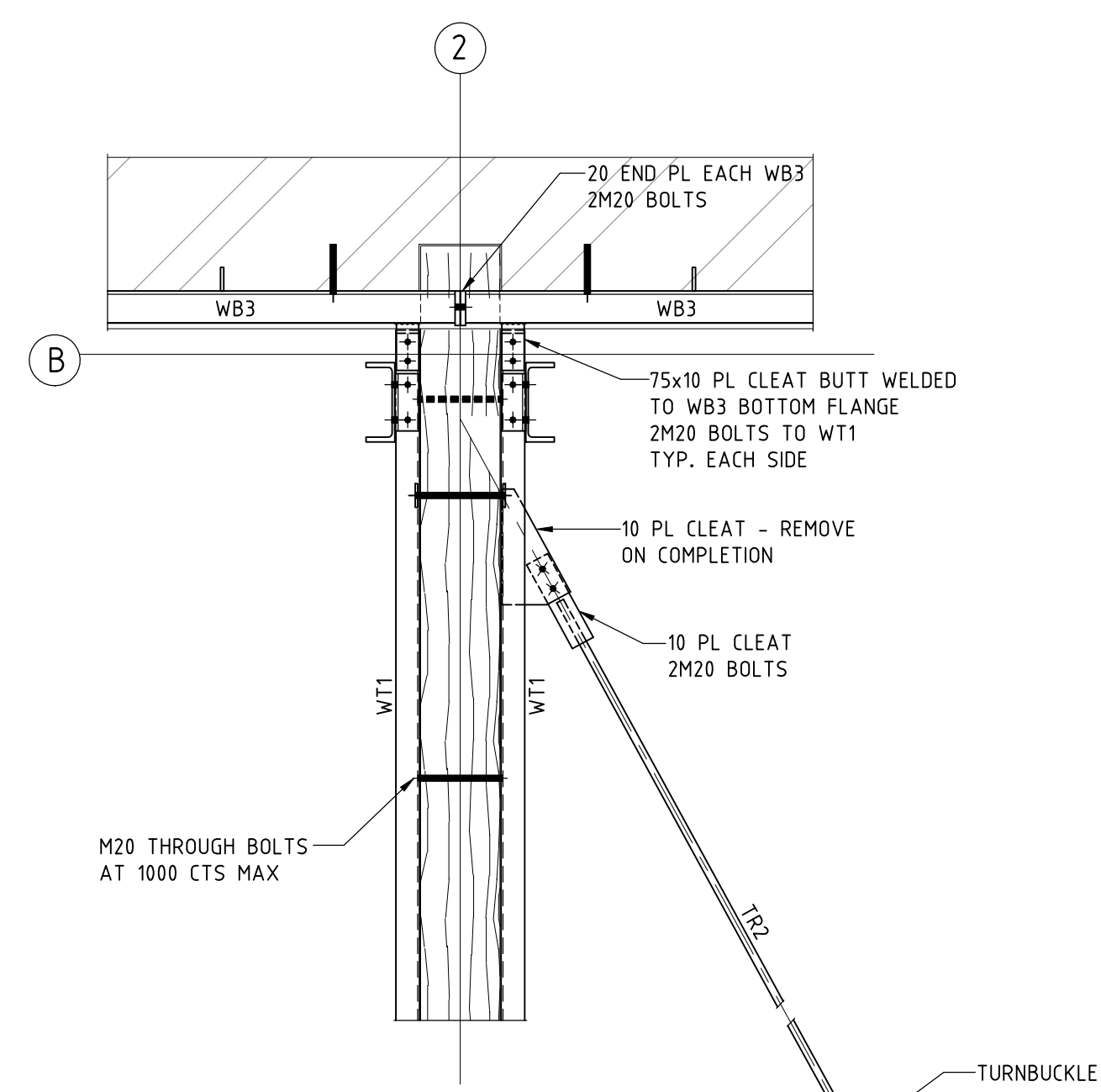
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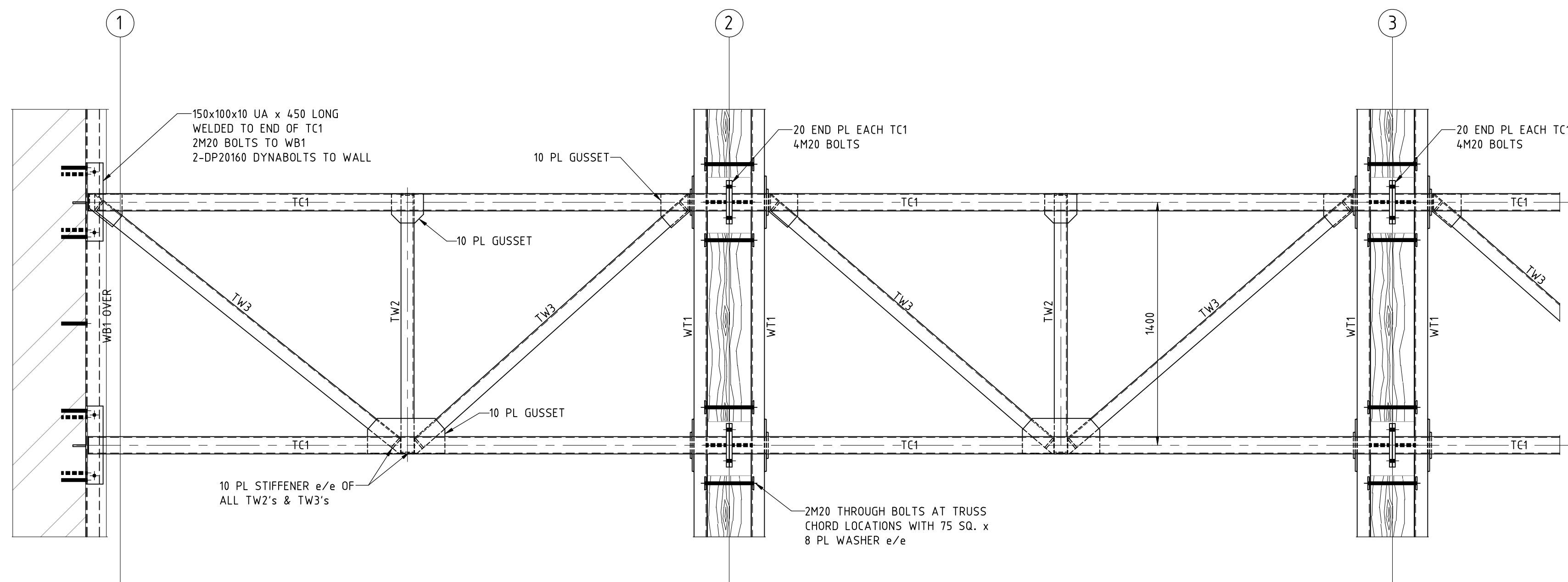
PLAN OF OUTRIGGER WEST END



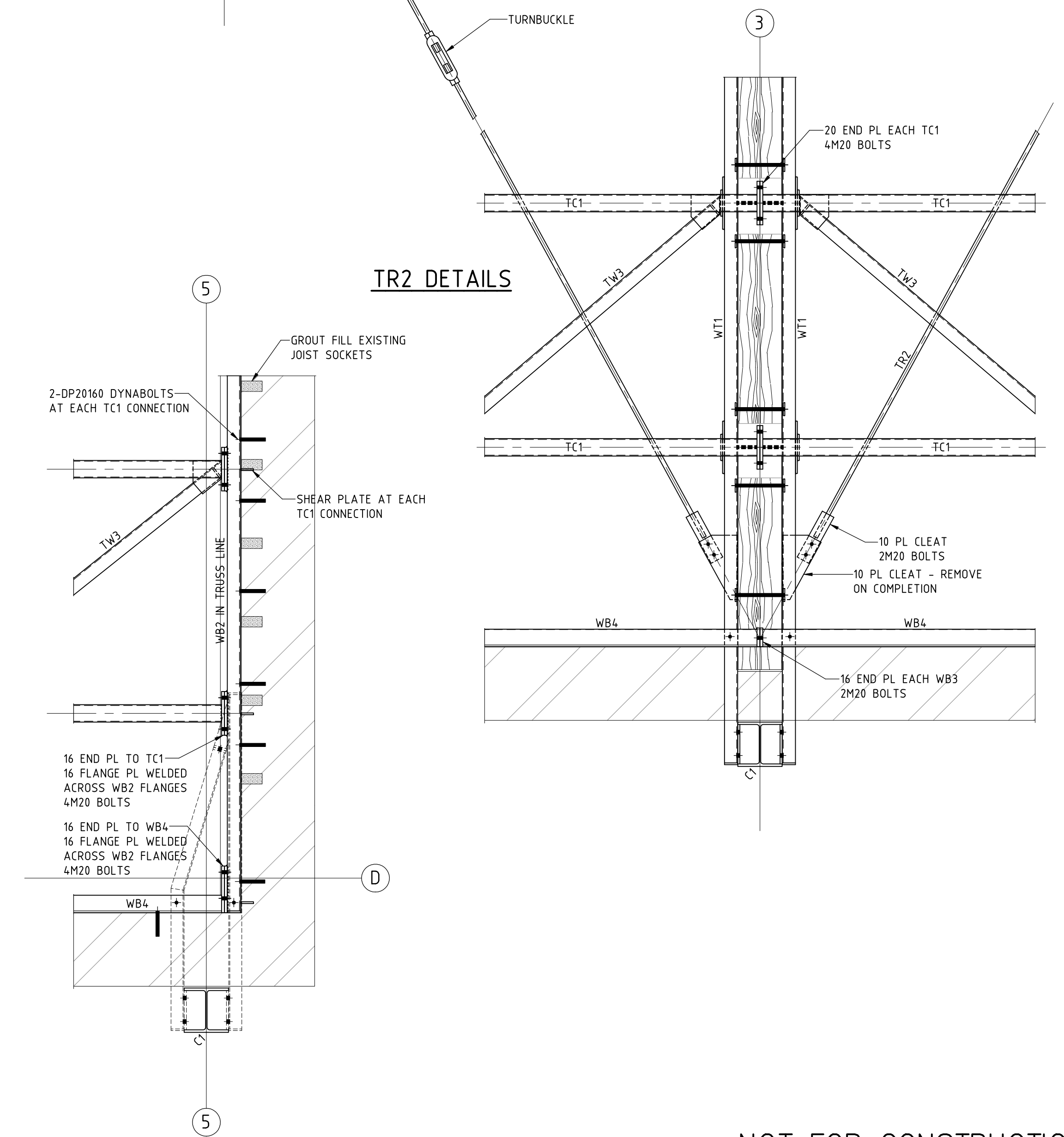
PLAN OF OUTRIGGER EAST END

TR1 DETAIL
OTHER END SIMILAR TO TR2

TR2 DETAILS



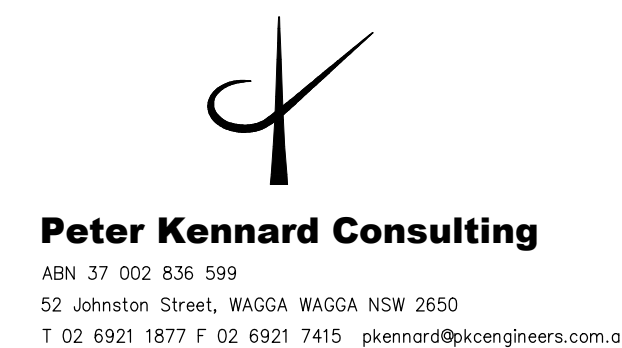
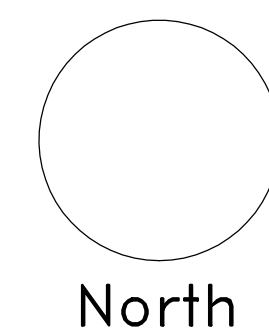
STIFFENING TRUSS DETAILS



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Revision	ISSUED FOR INFORMATION	30.3.20	C.A.	P.J.K.	X	
	Amendment or reason for issue	Issue date	Designed & checked by	Designed & checked by	Verified by	Issue authorised (*)

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Project	OLD MILL GALLERY GUNDAGAI STAGE 1 - RECTIFICATION	Drawing Title	STEELWORK DETAILS SHEET 3
Client	COOTAMUNDRA GUNDAGAI REGIONAL COUNCIL	Scales	1:20
Architect / Project Manager	NOEL THOMSON ARCHITECTURE	Drawing No.	19S023-S09
		Client Project No.	
		Sheet	9 of 9
		Revision	-